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ALTIS BULKY RETAIL LTD AS TRUSTEE FOR ALTIS ARET SUB TRUST 20 (“ALTIS”)

Additional Geotechnical Investigation

28 Elizabeth Street, Liverpool NSW

E24175.G04
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1. Introduction

1.1 Background

At the request of Altis Bulky Retail Ltd as trustee for Altis ARET Sub Trust 20 ("Altis") (the Client), EI Australia (EI) has carried out an Additional Geotechnical Investigation (AGI) for the proposed development at 28 Elizabeth Street, Liverpool NSW (the Site).

This AGI report has been prepared to provide advice and recommendations to assist in the preparation of designs for the proposed development. The investigation has been carried out in accordance with the agreed scope of works outlined in EI's proposal referenced P18745.3, dated 22 January 2021, and with the Client's signed authorisation to proceed, dated 8 June 2021.

EI has previously completed a Geotechnical Investigation (GI) Factual Report, referenced E24175.G03, dated 22 May 2019. This AGI report should be read in conjunction with the referenced GI report.

1.2 Proposed Development

The following documents, supplied by the Client, were used to assist with the preparation of this AGI report:

- Massing Study prepared by Turner – Project No. 20089, dated 26 November 2020; and
- Architectural Plans titled "Illoura Place" prepared by Turner – Project No. 20089, Drawing Nos. DA-001-001, DA-010-010 to DA-010-013, DA-110-001 to DA-110-019, DA-110-120 to DA-110-123, DA-110-330, DA-110-340, DA-210-101 to DA-210-401, DA-310-101, DA-310-102, DA-310-201, and DA-310-202, Revision S1, dated 20 October 2021.

Based on the provided scope of work and discussion, EI understands that the development will comprise a thirty-four-storey mixed-use building overlying a six-level basement. The lowest basement level is proposed to have a finished floor level (FFL) of RL -5.45m. A Bulk Excavation Level (BEL) of RL -5.70m is assumed, which includes allowance for the construction of the basement slab. To achieve the BEL, excavation depths from 18.8m to 19.7m Below Existing Ground Level (BEGL) have been estimated. Locally deeper excavations may be required for footings, lift overrun pits, crane pads, and service trenches. The basement extends up to the northern, western, southern, and eastern boundaries.

1.3 Objectives

The objective of the AGI was to assess site surface and subsurface conditions by drilling deeper boreholes to understand the nature and the strength of the shale bedrock at similar locations to previous GI, and to provide geotechnical advice and recommendations addressing the following:

- Dilapidation Surveys;
- Excavation methodologies and monitoring requirements;
- Groundwater considerations;
- Vibration considerations;

- Excavation support requirements, including preliminary geotechnical design parameters for retaining walls and shoring systems;
- Building foundation options, including;
  - Design parameters.
  - Earthquake loading factor in accordance with AS1170.4:2007.

1.4 Scope of Works

The scope of works for the AGI included:

- Preparation of a Work Health and Safety Plan;
- Review of previous GI report by EI and relevant geological maps for the project area;
- Site walkover inspection by a Geotechnical Engineer to assess topographical features and site conditions;
- Scanning of proposed borehole locations for buried conductive services using a licensed service locator with reference to Dial Before You Dig (DBYD) plans;
- Drilling of five boreholes (BH101, BH102M, BH103, BH104 and BH105) by a track-mounted drill rig using solid flight augers equipped with a 'Tungsten-Carbide' (T-C) bit. The boreholes were auger drilled, continued using NMLC diamond coring techniques to termination depths as shown in **Table1-1** below. The rock core photographs are presented in **Appendix A**;

Table 1-1 Augering, Wash Boring and Rock Coring Depths

Borehole ID	Augering		Wash Boring		Rock Coring	
	Depth (m)	RL (m AHD)	Depth (m)	RL (m AHD)	Depth (m)	RL (m AHD)
BH101	5.50	7.56	16.55	-3.49	22.00	-8.94
BH102M	7.00	6.24	16.60	-3.36	23.00	-9.76
BH103	5.50	7.58	16.60	-3.52	20.72	-7.64
BH104	7.50	6.26	16.60	-2.84	21.00	-7.24
BH105	5.50	8.46	16.50	-2.54	22.33	-8.37

  - Measurements of groundwater seepage/levels, where possible, in the augered sections of the boreholes during and shortly after completion of auger drilling;
  - The strength of the bedrock in the augered sections of the boreholes was assessed by observation of the auger penetration resistance using a T-C drill bit and examination of the recovered rock cuttings. It should be noted that rock strengths assessed from augered boreholes are approximate and strength variances can be expected.
  - The approximate surface levels shown on the borehole logs were interpolated from previous GI completed by EI. Approximate borehole locations are shown on **Figure 2**;
- Borehole BH102M was converted into a groundwater monitoring well with a depth of 22.9m BEGL (RL -9.66m) to allow for long-term groundwater monitoring.

- Boreholes BH101, BH103, BH104 and BH105 were backfilled with drilling spoils and capped upon completion;
- Rock samples were sent to STS Geotechnics Pty Ltd (STS) which is National Australian Testing Authority (NATA) accredited laboratories, for testing and storage.
- Preparation of this AGI report.

EI's Geotechnical Engineer was present full-time onsite to set out the borehole locations, direct the testing and sampling, log the subsurface conditions and record groundwater levels.

1.5 Constraints

The AGI was limited by the intent of the investigation and the presence of existing site structures. The discussions and advice presented in this report are intended to assist in the preparation of final designs for the proposed development. Further geotechnical inspections should be carried out during construction to confirm the geotechnical and groundwater models, and the design parameters provided in this report.

2. Site Description

2.1 Site Description and Identification

The site identification details and associated information are presented in **Table 2-1** below while the site locality is shown on **Figure 1.** An aerial photograph of the site is presented in **Plate 1** below. The site conditions appeared to be similar to the previous GI dated 2019. Therefore the site descriptions from the previous GI report has been reproduced.

Table 2-1 Summary of Site Information

Information	Detail
Street Address	28 Elizabeth Street, Liverpool NSW
Lot and Deposited Plan (DP) Identification	Lot 1 in DP 516633, Lot 2 and 3 in DP 700219, and Lot 4 in DP 592346
Brief Site Description	At the time of our investigation all site structures had been demolished. The northeast corner of the site, which had previously been a petrol station, was still concrete paved and had the slab of the original structure remaining. This area is raised up to 1m above the natural ground level on the east, and is on grade with street level on the west. A number of underground storage tanks (USTs) were also identified in this area, particularly towards the northern boundary. The northwest corner of the site was concrete and bitumen paved and had once been used for parking. The southern half of the site had a gravel base and was overgrown, particularly in the south east. Site access was via a concrete paved driveway towards the eastern edge of the site along Elizabeth street. The site north, south and west boundaries had permanent/temporary fencing. A low to medium height brick retaining wall was constructed along the southern site boundary for the neighbour's driveway.
Site Area	The site area is approximately 3,609m^2 (based on architectural drawings).

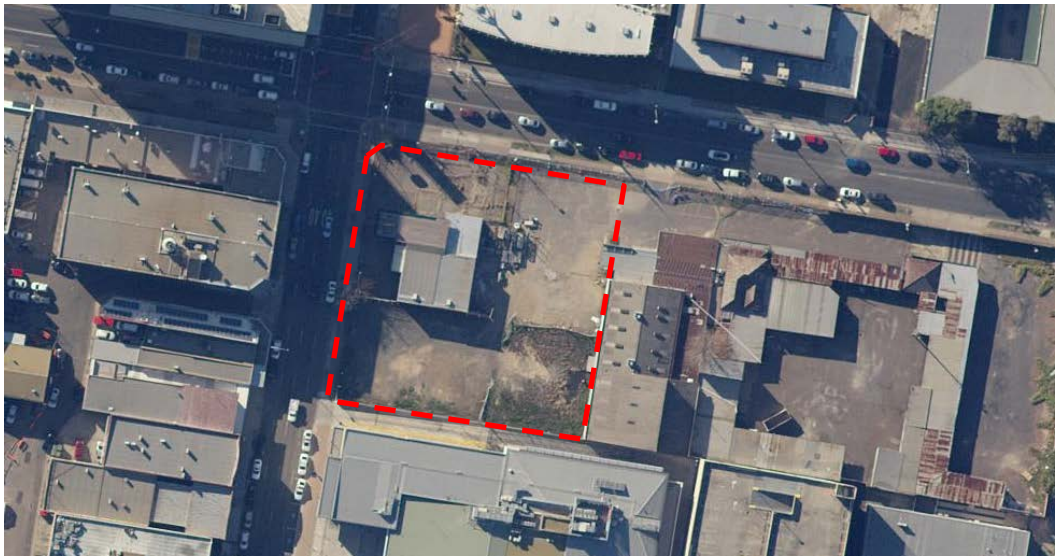


Plate 1: Aerial photograph of the site (source: SIX Maps, accessed 13/7/21)

2.2 Local Land Use

The site is situated within an area of mixed use. Current uses on surrounding land at the time of our presence on site are described in **Table 2-2** below. For the sake of this report, the site boundary adjacent to Elizabeth Street shall be adopted as the northern site boundary.

Table 2-2 Summary of Local Land Use

Direction Relative to Site	Land Use Description
North	Elizabeth Street, a four lane, asphalt-paved road. Beyond this is a grassed lawn (retained by an approx. 0.5m high brick retaining wall), followed by an approx. three-storey tall church building set back about 26m from the northern site boundary.
East	No. 26 Elizabeth Street, a concrete paved block with all previous site structures demolished.
South	The Liverpool Police Station, a three-storey brick building set back approx. 6m from the southern site boundary. It is unknown if a basement is present. The building appears to be in good condition.
West	George Street, a four lane, asphalt-paved road, followed by two to four-storey mixed use buildings with ground floor commercial tenancies.

2.3 Regional Setting

The site topography and geological information for the locality is summarised in **Table 2-3** below**.**

Table 2-3 Topographic and Geological Information

Attribute	Description
Topography	The site is generally flat and level with site surface levels between RL 13.96m at the north-eastern end and RL 13.08m at the south western end.
Regional Geology	Information on regional sub-surface conditions, referenced from the Department of Mineral Resources Geological Map Penrith 1:100,000 Geological Series Sheet 9030 (DMR 1991) indicates the site is underlain by Bringelly Shale. Bringelly Shale typically comprises shale, carbonaceous claystone, claystone, laminite, fine to medium-grained lithic sandstone, rare coal and tuff.

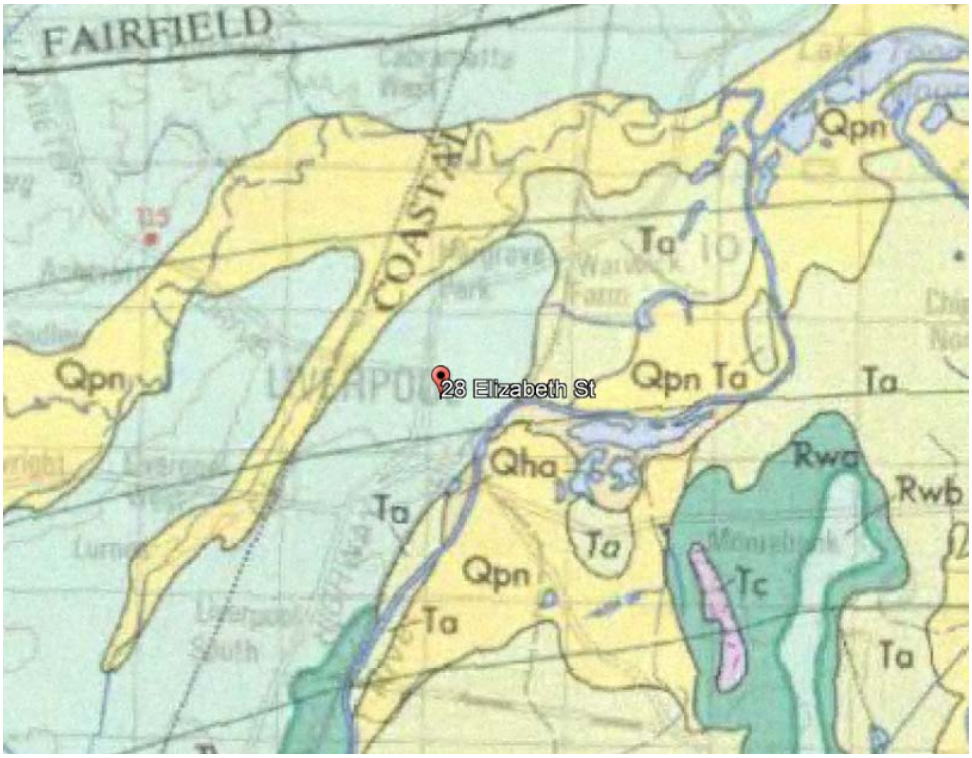


Plate 2: Excerpt of geological map showing location of site.

3. Investigation Results

3.1 Stratigraphy

For the development of a site-specific geotechnical model, the stratigraphy observed in the GI has been grouped into four geotechnical units. A summary of the subsurface conditions across the site, interpreted from the assessment results which also include the findings in EI's previous GI (Reference E24175.G03 dated 22 May 2019), is presented in **Table 3-1** below. More detailed descriptions of subsurface conditions at each borehole location are available on the borehole logs presented in **Appendix A**. The details of the methods of soil and rock classifications, explanatory notes and abbreviations adopted on the borehole logs are also presented in **Appendix A.**

Table 3-1 Summary of Subsurface Conditions

Unit	Material[2]	Depth to Top of Unit (m BEGL)[1]	RL of Top of Unit (m AHD)[1]	Observed Thickness (m)	Comments
1	**Fill**	Surface to 0.12	12.95 to13.84	0.55 to 1.0	Concrete pavement of 100mm to 120mm thickness, asphalt of 30mm thickness, or gravel present at surface, underlain by Fill consisting of gravelly to silty sand, and clayey sand to clay, with gravels and/or brick fragments, and trace ash. Fill was assessed based on our observations during drilling and SPT N Values (from EI's previous GI) to be poorly compacted.
2	**Residual Soil**	0.55 to 1.0	12.08 to13.16	2.98 to 5.95	Very soft to very stiff silty clay of medium to high plasticity becoming Hard at depths, with rare ironstone bands, grading to extremely weathered shale/sandstone.
3	**Very Low to Low Strength Shale/Sandstone**	3.78 to 6.5	6.58 to 9.76	0.96 to 2.7	Distinctly weathered, very low to low strength sandstone/shale with occasional clay seams.
4	**Medium to High Strength Shale**	4.74 to 9.0	4.06 to -8.31	-[3]	Fresh, medium to high strength shale interbedded with sandstone.

Note 1 Approximate depth and level at the time of our assessment. Depths and levels may vary across the site.
Note 2 For more detailed descriptions of the subsurface conditions, reference should be made to the borehole logs attached to **Appendix A.**
Note 3 Observed up to termination depth in all boreholes.

3.2 Groundwater Observations

Groundwater seepage was not observed during auger drilling of boreholes in this AGI. Water circulation due to coring within the boreholes prevented further observations of groundwater levels within all boreholes. We note that the groundwater levels may not have become evident or stabilised in the augered boreholes within the limited observation period. No long term groundwater monitoring was carried out.

Groundwater levels were measured in the existing groundwater monitoring wells on site, no borehole reference was available at time of AGI. The groundwater levels were observed at depths ranged between 3.13m BEGL (or RL9.48 AHD) and 4.48m BEGL (RL10.11m AHD).

3.3 Test Results

29 selected rock core samples were tested by STS in this AGI to estimate the Point Load Strength Index (Is_{50}) values to assist with rock strength assessment. The results of the testing are summarised on the attached borehole logs.

The point load strength index tests correlated reasonably well with our field assessments of rock strength. The approximate Unconfined Compressive Strength (UCS) of the rock core, estimated from correlations with the point load strength index test results, varied from 11 MPa to 54 MPa.

4. Recommendations

4.1 Geotechnical Issues

Based on the results of the investigation, we consider the following to be the main geotechnical issues for the proposed development:

- Basement excavation and retention to limit lateral deflections and ground loss as a result of excavations, resulting in damage to nearby structures;
- Rock excavation; and
- Foundation design for building loads.

4.2 Dilapidation Surveys

Prior to excavation and construction, we recommend that detailed dilapidation surveys be carried out on all structures and infrastructures surrounding the site that falls within the zone of influence of the excavation to allow assessment of the recommended vibration limits and protect the client against spurious claims of damage. The zone of influence of the excavation is defined by a distance back from the excavation perimeter of twice the total depth of the excavation. The reports would provide a record of existing conditions prior to commencement of the work. A copy of each report should be provided to the adjoining property owner who should be asked to confirm that it represents a fair assessment of existing conditions. The reports should be carefully reviewed prior to demolition and construction.

4.3 Excavation Methodology

4.3.1 Excavation Assessment

Prior to any excavation commencing, we recommend that reference be made to the Safe Work Australia Excavation Work Code of Practice, dated January 2020.

EI assumes that the proposed development may require an excavation depth of from 18.8m to 19.7m BEGL. Locally deeper excavations for footings, service trenches, crane pads and lifts overrun pits may be required.

Based on the borehole logs, the proposed basement excavations will therefore extend through all units as outlined in **Table 3-1** above. As such, an engineered retention system must be installed prior to excavation commencing.

Units 1 and 2 could be excavated using buckets of large earthmoving Hydraulic Excavators, particularly if fitted with ‘Tiger Teeth’. Excavation of Units 3 and 4 may present hard or heavy ripping, or “hard rock” excavation conditions. Ripping would require a high capacity and heavy bulldozer for effective production. Wear and tear should also be allowed for. The use of a smaller size bulldozer will result in lower productivity and higher wear and tear, and this should be allowed for. Alternatively, hydraulic rock breakers, rock saws, ripping hooks or rotary grinders could be used, though productivity would be lower and equipment wear increased, and this should be allowed for.

Should rock hammers be used for the excavation of the bedrock, excavation should commence away from the adjoining structures and the transmitted vibrations monitored to assess how close the hammer can operate to the adjoining structures while maintaining transmitted vibrations within acceptable limits. To fall within these limits, we recommend that the size of rock hammers do not exceed a medium sized rock hammer, say 900 kg, such as a Krupp 580, and

be trialled prior to use. The transmitted vibrations from rock hammers should be measured to determine how close each individual hammer can operate to the adjoining buildings.

The vibration measurements can be carried out using either an attended or an unattended vibration monitoring system. An unattended vibration monitoring system must be fitted with an alarm in the form of a strobe light or siren or alerts sent directly to the site supervisor to make the plant operator aware immediately when the vibration limit is exceeded. The vibration monitor must be set to trigger the alarm when the overall Peak Particle Velocity (PPV) exceeds set limits outlined by a vibration monitoring plan. Reference should be made to **Appendix C** for a guide to acceptable limits of transmitted vibrations.

If it is found that the transmitted vibrations by the use of rock hammers are unacceptable, then it would be necessary to change to a smaller excavator with a smaller rock hammer, or to a rotary grinder, rock saws, jackhammers, ripping hooks, chemical rock splitting and milling machines. Although these are likely to be less productive, they would reduce or possibly eliminate risks of damage to adjoining properties through vibration effects transmitted via the ground. Such equipment would also be required for detailed excavation, such as footings or service trenches, and for trimming of faces. Final trimming of faces may also be completed using a grinder attachment rather than a rock breaker in order to assist in limiting vibrations. The use of rotary grinders generally generates dust and this may be supressed by spraying with water.

To assist in reducing vibrations and over-break of the bedrock, we recommend that initial saw cutting of the excavation perimeters through the bedrock may be provided using rock saw attachments fitted to the excavator. Rock sawing of the excavation perimeter has several advantages as it often reduces the need for rock bolting as the cut faces generally remain more stable and require a lower level of rock support than hammer cut excavations, ground vibrations from rock saws are minimal and the saw cuts will provide a slight increase in buffer distance for use of rock hammers. However, the effectiveness of such approach must be confirmed by the results of vibration monitoring.

Also, there is a potential for poorly oriented defects within the excavated bedrock to result in localized rock slide/topple failure with potential impact to the work site or the adjacent structures. However through selection of suitable excavation equipment, geotechnical inspections and mapping during the excavation works along with the installation of support measures as determined necessary by the inspections, the risk from the proposed works can be maintained within ‘Acceptable’ levels. In addition, we recommend that only excavation contractors with appropriate insurances and experience on similar projects be used. The contractor should also be provided with a copy of this report to make his own judgement on the most appropriate excavation equipment.

Groundwater seepage monitoring should be carried out during bulk excavation works and prior to finalising the design of a pump out facility. Outlets into the stormwater system will require Council approval.

Furthermore, any existing buried services, which run below the site, will require diversion prior to the commencement of excavation or alternatively be temporarily supported during excavation, subject to permission or other instructions from the relevant service authorities. Enquiries should also be made for further information and details, such as invert levels, on the buried services.

4.3.2 Excavation Monitoring

Consideration should be made to the impact of the proposed development upon neighbouring structures, roadways and services. Basement excavation retention systems should be designed so as to limit lateral deflections.

Contractors should also consider the following limits associated with carrying out excavation and construction activities:

- Limit lateral deflection of temporary or permanent retaining structures;
- Limit vertical settlements of ground surface at common property boundaries and services easement; and
- Limit Peak Particle Velocities (PPV) from vibrations, caused by construction equipment or excavation, experienced by any nearby structures and services.

Monitoring of deflections of retaining structures and surface settlements should be carried out by a registered surveyor at agreed points along the excavation boundaries and along existing building foundations / services / pavements and other structures located within or near the zone of influence of the excavation. Owners of existing services adjacent to the site should be consulted to assess appropriate deflection limits for their infrastructures. Measurements should be taken in the following sequence:

- Before commencing installation of retaining structures where appropriate to determine the baseline readings. Two independent sets of measurements must be taken confirming measurement consistency;
- After installation of the retaining structures, but before commencement of excavation;
- After excavation to the first row of supports or anchors, but prior to installation of these supports or anchors;
- After excavation to any subsequent rows of supports or anchors, but prior to installation of these supports or anchors;
- After excavation to the base of the excavation;
- After de-stressing and removal of any rows of supports or anchors; and
- One month after completion of the permanent retaining structure or after three consecutive measurements not less than a week apart showing no further movements, whichever is the latter.

4.4 Groundwater Considerations

Groundwater seepage was not observed during auger drilling of boreholes in this AGI

Based on the low permeability of the bedrock profile any groundwater inflows into the excavation should not have an adverse impact on the proposed development or on the neighbouring sites and should be manageable. However, we expect that some groundwater inflows into the excavation along the soil/rock interface and through any defects within the sandstone/ shale bedrock (such as jointing, and bedding planes, etc.) particularly following a period of heavy rainfall. The initial flows into the excavation may be locally high, but would be expected to decrease considerably with time as the bedding seams/joints are drained. We recommend that monitoring of seepage be implemented during the excavation works to confirm the capacity of the drainage system.

We expect that any seepage that does occur will be able to be controlled by a conventional sump and pump system. We recommend that a sump-and-pump system be used both during construction and for permanent groundwater control below the basement floor slab.

In the long term, drainage should be provided behind all basement retaining walls, around the perimeter of the basement and below the basement slab. The completed excavation should be inspected by the hydraulic engineer to confirm that adequate drainage has been allowed for. Drainage should be connected to the sump-and-pump system and discharging into the stormwater system. The permanent groundwater control system should take into account any possible soluble substances in the groundwater which may dictate whether or not groundwater can be pumped into the stormwater system.

4.5 Excavation Retention

4.5.1 Support Systems

From a geotechnical perspective, it is critical to maintain the stability of all adjacent structures and infrastructures during demolition, excavation and construction works.

Based on the depth of the excavation, the encountered subsurface conditions and limited setbacks, temporary batters are not recommended for this site. Based on the above, the close proximity of the surrounding buildings, the encountered subsurface conditions, the shallow groundwater, and the required excavation depth, temporary batters are not recommended for this site. Unsupported vertical cuts of the soil are not recommended for this site as these carry the risk of potential collapse especially after a period of wet weather. Collapse of the material may result in injury to personnel and/or damage to nearby structures/infrastructures and equipment.

A suitable retention system will be required for the support units 1, 2 and 3. For this site, EI recommends an anchored and/or propped soldier pile wall with mass concrete in between the piles be founded into medium to high strength shale (Unit 4). Consideration may be made for some shoring piles, which are not supporting the vertical structural loads of the building, to be terminated at least 0.5m, into Unit 4 material or better, above the base of the bulk excavation levels. Excavation within Unit 4 shale should generally be able to be cut vertically and without support, provided an anchor is installed at the toe of the solider pile wall. Anchors/props and mass concrete must be installed progressively as excavation proceeds. The contiguous/secant pile wall may be considered as an alternative as this will provide sufficient rigidity to support deep excavations and also assist in supporting the poorly oriented defects within the excavated shale bedrock**.**

We highly recommend increasing the shoring pile lengths in BH4 location where these defects extend to depth of 9m BEGL or increase the length of every fourth pile and provide shotcrete panels to stabilize these poorly oriented defects.

Details of nearby basements, shoring pile walls and anchors must be obtained prior to final design. The installation of anchors may possibly be restricted by the presence of the basement structures adjacent to the site, anchors installation may not be possible and internal props may be required.

For vertical cuts, the excavations must be inspected by a geotechnical engineer at regular intervals to check for any inclined joints or weak seams that require stabilisation. Such geotechnical inspections should be carried out at depth intervals of no more than 1.5m. If adverse defects are encountered, the stabilisation measures may comprise rock bolts, shotcrete and mesh or dental treatment of thin weak seams using non-shrink grout, and this should be allowed for.

The existence of significant horizontal in-situ stresses in bedrock, particularly in the Sydney basin, is well established. The release of such stresses during the basement excavation may cause adverse impact on the stability of the excavation faces and thus increase the movements. Monitoring of several deep excavations within sandstone and shale in the Sydney region indicates that the lateral displacement at the top of the excavation is generally between 0.5mm to 2mm per meter depth of excavation. As the maximum depth of excavation is of about 17m, a lateral deflection at the crest of the excavation up to 34mm can be expected which will reduce in a stepped fashion to zero at the bulk excavation level. Monitoring of the lateral movement as the excavation progresses is recommended. An assessment of such movements and their impact can be carried out using finite element software such as PLAXIS.

Bored piles are considered to be the most suitable for this site. Tremie pumps may be required where high groundwater seepage inflows are present during the drilling of the bored piles. However, relatively large capacity piling rigs will be required for drilling through the bedrock. The proposed pile locations should take into account the presence of buried services. Further advice should be sought from prospective piling contractors who should be provided with a copy of this report.

4.5.2 Retaining Wall Design Parameters

The following parameters may be used for static design of temporary and permanent retaining walls at the subject site:

- For progressively anchored or propped walls where minor movements can be tolerated (provided there are no buried movement sensitive services), we recommend the use of a trapezoidal earth pressure distribution of 6H kPa for soil, where H is the retained height in meters. These pressures should be assumed to be uniform over the central 50% of the support system, tapering to nil at top and bottom;
- For progressively anchored or propped walls which support areas which are highly sensitive to movement (such as areas where movement sensitive structures or infrastructures or buried services are located in close proximity), we recommend the use of a trapezoidal earth pressure distribution of 8H kPa for soil, where ‘H’ is the retained height in meters. These pressures should be assumed to be uniform over the central 50% of the support system, tapering to nil at top and bottom;
- All surcharge loading affecting the walls (including from construction equipment, construction loads, adjacent high level footings, etc.) should be adopted in the retaining wall design as an additional surcharge using an ‘at rest’ earth pressure coefficient, Ko.
- The retaining walls should be designed as drained and measures are to be taken to provide complete and permanent drainage behind the walls. Strip drains protected with a non-woven geotextile fabric should be used behind the shotcrete infill panels for soldier pile walls.
- For piles embedded into Unit 4 or better, the allowable lateral toe resistance values outlined in **Table 4-1** below may be adopted. These values assume excavation is not carried out within the zone of influence of the wall toe and the rock does not contain adverse defects etc. The upper 0.3m depth of the socket should not be taken into account to allow for tolerance and disturbance effects during excavation.
- If temporary anchors extend beyond the site boundaries, then permission from the neighbouring properties would need to be obtained prior to installation. Also, the presence of neighbouring basements and/or services and their levels must be confirmed prior to finalising anchor design**.**

- Anchors should have their bond length within Unit 3 or better. For the design of anchors bonded into Unit 3 or better, the allowable bond stress value outlined in **Table 4-1** below may be used, subject to the following conditions:

  1. Anchor bond lengths of at least 3m behind the 'active' zone of the excavation (taken as a 45 degree zone above the base of the excavation) is provided;
  2. Overall stability, including anchor group interaction, is satisfied;
  3. All anchors should be proof loaded to at least 1.33 times the design working load before locked off at working load. Such proof loading is to be witnessed by and engineer independent of the anchoring contractor. We recommend that only experienced contractors be considered for anchor installation with appropriate insurances;
  4. If permanent anchors are to be used, these must have appropriate corrosion provisions for longevity.

Table 4-1 Geotechnical Design Parameters

Material [1]		Unit 1 Fill	Unit 2 Residual Soil	Unit 3 Very Low Strength Sandstone / Shale	Unit 4 Medium to high Strength Shale
RL of Top of Unit (m AHD) [2]		**12.95 to 13.84**	**12.08 to 13.16**	**6.58 to 9.76**	**4.06 to -8.31**
Bulk Unit Weight (kN/m^3)		**18**	**20**	**23**	**24**
Friction Angle, ϕ' (°)		**25**	**25**	**34**	**40**
Earth Pressure Coefficients	At rest, K_o [3]	0.58	0.58	0.43	-
	Active, K_a [3]	0.41	0.41	0.27	-
	Passive, K_p [3]	-	-	3.69	-
Allowable Bearing Pressure (kPa) [5]		-	-	700	3500
Allowable Shaft Adhesion (kPa) [4, 5]	in Compression	-	-	70	350
	in Uplift	-	-	35	175
Allowable Toe Resistance (kPa)		-	-	70	350
Allowable Bond Stress (kPa)		-	-	50	350
Earthquake Site Risk Classification		▪ AS 1170.4:2007 indicates an earthquake subsoil class of Class C_e (Shallow Soil) ▪ AS 1170.4:2007 indicates that the hazard factor (z) for Sydney is 0.08.			

Notes:

1 More detailed descriptions of subsurface conditions are available on the borehole logs presented in **Appendix A.**

2 Approximate levels of top of unit at the time of our investigation. Levels may vary across the site.

3 Earth pressures are provided on the assumption that the ground behind the retaining walls is horizontal.

4 Side adhesion values given assume there is intimate contact between the pile and foundation material and should achieve a clean socket roughness category R2 or better. Design engineer to check both 'piston pull-out' and 'cone liftout' mechanics in accordance with AS4678-2002 Earth Retaining Structures.

5 To adopt these parameters we have assumed that:

- Footings have a nominal socket of at least 0.3m, into the relevant founding material;
- For piles, there is intimate contact between the pile and foundation material (a clean socket roughness category of R2 or better);
- Potential soil and groundwater aggressivity will be considered in the design of piles and footings;
- Piles should be drilled in the presence of a Geotechnical Engineer prior to pile construction to verify that ground conditions meet design assumptions. Where groundwater ingress is encountered during pile excavation, concrete is to be placed as soon as possible upon completion of pile excavation. Pile excavations should be pumped dry of water prior to pouring concrete, or alternatively a tremmie system could be used;
- The bases of all pile, pad and strip footing excavations are cleaned of loose and softened material and water is pumped out prior to placement of concrete;
- The concrete is poured on the same day as drilling, inspection and cleaning.
- The allowable bearing pressures given above are based on serviceability criteria of settlements at the footing base/pile toe of less than or equal to 1% of the minimum footing dimension (or pile diameter).

4.6 Foundations

Following bulk excavation to RL -5.70m, we expect Unit 4 'Medium to High Strength Shale' bedrock to be exposed at BEL.

It is recommended that all footings for the building be founded within the shale bedrock of similar strength of at least Unit 4 or better to provide uniform support and reduce the potential for differential settlements.

Pad or strip footings founded within Unit 4 may be designed for an allowable bearing capacity of 3500kPa, based on serviceability.

Geotechnical inspections of foundations are recommended to determine that the required bearing capacity has been achieved and to determine any variations that may occur between the boreholes and inspected locations.

Alternatively, the proposed development may be supported on deep foundations if heavy loads are anticipated, such as piles founded into shale bedrock (Unit 4).

For piles founded shale bedrock, these must be embedded a minimum of 0.5m into shale, and can be designed for a maximum allowable bearing pressure of 3500kPa. The allowable shaft adhesion in shale bedrock may be designed as 10% of the allowable bearing pressure (or 5% for uplift) for the socket length in excess of 0.5m.

At least the initial drilling of piles should be completed in the presence of a geotechnical engineer to verify that ground conditions meet design assumptions.

Where groundwater ingress is encountered during pile excavation, concrete is to be placed as soon as possible upon completion of pile excavation. Pile excavations should be pumped dry of water prior to pouring concrete, or alternatively a tremmie system could be used. Concrete must be poured on the same day as drilling, inspection and drilling.

4.7 Basement Floor Slab

Following bulk excavations for the proposed basement, shale bedrock is expected to be exposed at the basement floor BEL.

Following the removal of all loose and softened materials, we recommend that underfloor drainage be provided and should comprise a strong, durable, single sized washed aggregate such as ‘blue metal gravel’. Joints in the concrete floor slab should be designed to accommodate shear forces but not bending moments by using dowelled and keyed joints. The basement floor slab should be isolated from columns. The completed excavation should be inspected by the hydraulic engineer to confirm the extent of the drainage required.

In addition, a system of sub-soil drains comprising a durable single sized aggregate with perforated drains/pipes leading to sumps should be provided. The basement floor slab should be isolated from columns.

Permission may need to be obtained from the NSW Department of Primary Industries (DPI) and possibly Council for any permanent discharge of seepage into the drainage system. Given the subsurface conditions, we expect that seepage volumes would be low and within the DPI limits. However, if permission for discharge is not obtained, the basement may need to be designed as a tanked basement.

5. Further Geotechnical Inputs

Below is a summary of the recommended additional work that needs to be carried out:

- Dilapidation surveys;
- Long term groundwater monitoring and seepage modelling for finalization of suitable retention system;
- Working Platform Assessment for piling rig, if required;
- Classification of all excavated material transported off site;
- Witnessing installation of support measures and proof-testing of anchors (if required).
- Geotechnical inspections of unsupported vertical excavations in bedrock;
- Geotechnical inspections of all new footings/piles by an experienced geotechnical professional before concrete or steel are placed to verify their bearing capacity and the in-situ nature of the founding strata; and
- Ongoing monitoring of groundwater inflows into the bulk excavation;

We recommend that a meeting be held after initial structural design has been completed to confirm that our recommendations have been correctly interpreted. We also recommend a meeting at the commencement of construction to discuss the primary geotechnical issues and inspection requirements.

6. Statement of Limitations

This report has been prepared for the exclusive use of Ms Fiona Beaverson and Altis Bulky Retail Ltd as trustee for Altis ARET Sub Trust 20 ("Altis") who is the only intended beneficiary of EI's work. The scope of the assessment carried out for the purpose of this report is limited to those agreed with Ms Fiona Beaverson and Altis Bulky Retail Ltd as trustee for Altis ARET Sub Trust 20 ("Altis")

No other party should rely on the document without the prior written consent of EI, and EI undertakes no duty, or accepts any responsibility or liability, to any third party who purports to rely upon this document without EI's approval.

EI has used a degree of care and skill ordinarily exercised in similar investigations by reputable members of the geotechnical industry in Australia as at the date of this document. No other warranty, expressed or implied, is made or intended. Each section of this report must be read in conjunction with the whole of this report, including its appendices and attachments.

The conclusions presented in this report are based on a limited investigation of conditions, with specific sampling and test locations chosen to be as representative as possible under the given circumstances.

EI's professional opinions are reasonable and based on its professional judgment, experience, training and results from analytical data. EI may also have relied upon information provided by the Client and other third parties to prepare this document, some of which may not have been verified by EI.

EI's professional opinions contained in this document are subject to modification if additional information is obtained through further investigation, observations, or validation testing and analysis during construction. In some cases, further testing and analysis may be required, which may result in a further report with different conclusions.

We draw your attention to the document "Important Information", which is included in **Appendix D** of this report. The statements presented in this document are intended to advise you of what your realistic expectations of this report should be. The document is not intended to reduce the level of responsibility accepted by EI, but rather to ensure that all parties who may rely on this report are aware of the responsibilities each assumes in so doing.

Should you have any queries regarding this report, please do not hesitate to contact EI.

References

AS1289.6.3.1:2004, *Methods of Testing Soils for Engineering Purposes,* Standards Australia.

AS1726:2017, *Geotechnical Site Investigations*, Standards Australia.

AS2159:2009, *Piling – Design and Installation*, Standards Australia.

AS3600:2009, *Concrete Structures,* Standards Australia

Safe Work Australia Excavation Work Code of Practice, dated January 2020 – WorkCover NSW

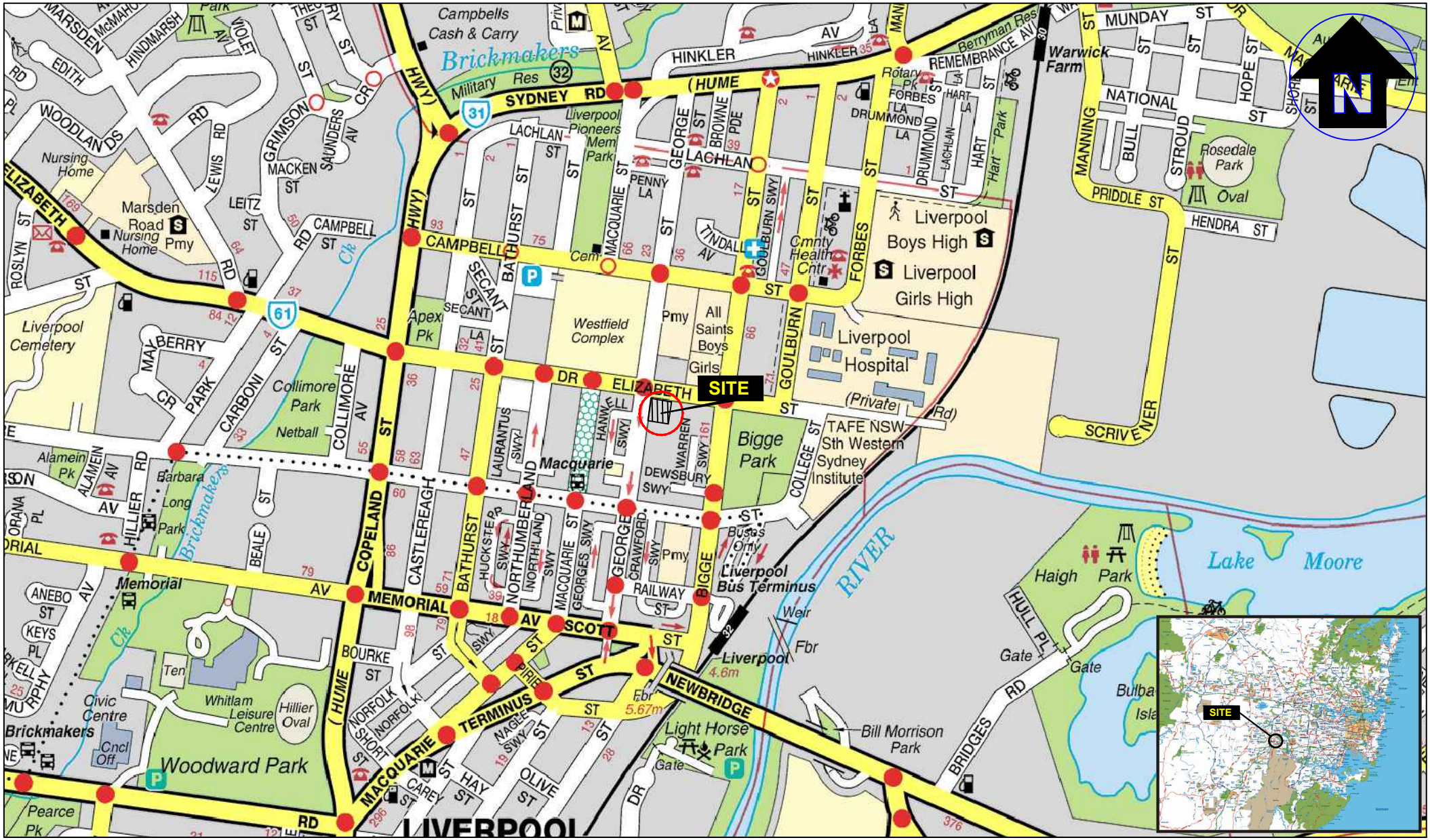
NSW Department of Finance and Service, Spatial Information Viewer, maps.six.nsw.gov.au.

NSW Department of Mineral Resources (1983) Sydney 1:100,000 Geological Series Sheet 9130 (Edition 1). Geological Survey of New South Wales, Department of Mineral Resources.

Abbreviations

AHD	Australian Height Datum
AS	Australian Standard
BEL	Bulk Excavation Level
BEGL	Below Existing Ground Level
BH	Borehole
DBYD	Dial Before You Dig
DP	Deposited Plan
EI	EI Australia
GI	Geotechnical Investigation
AGI	Additional Geotechnical Investigation
NATA	National Association of Testing Authorities, Australia
RL	Reduced Level
SPT	Standard Penetration Test
T-C	Tungsten-Carbide
UCS	Unconfined Compressive Strength

Figures



Drawn:	AM.H.	
Approved:	M.T.	
Date:	22-7-21	
Scale:	Not To Scale	

Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20

Additional Geotechnical Investigation
28 Elizabeth Street, Liverpool NSW
Site Locality Plan

Figure: 1

Project: E24175.G04

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Practical Solutions for Built Environments
Suite 6.01, 55 Miller Street, PYRMONT 2009
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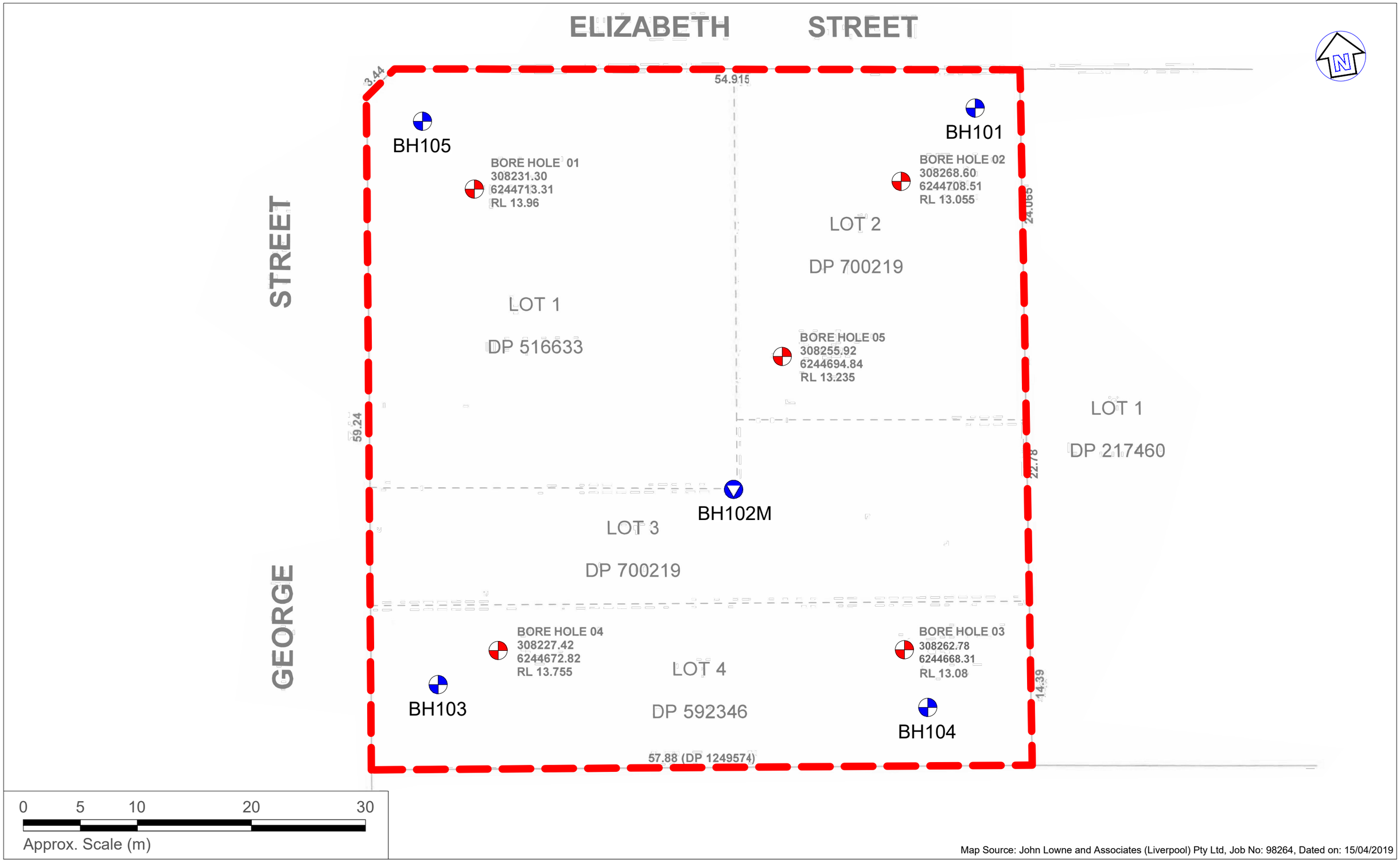

Map Source: John Lowne and Associates (Liverpool) Pty Ltd, Job No: 98264, Dated on: 15/04/2019

LEGEND (All Locations are Approximate)

- Site boundary
- Borehole location
- Borehole/monitoring well location
- Previous geotechnical investigation borehole location (EI, 2019)

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Drawn:	AM.H.
Approved:	M.T.
Date:	22-7-21

Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20
Additional Geotechnical Investigation
28 Elizabeth Street, Liverpool NSW
Borehole Location Plan

Figure: 2

Project: E24175.G04

Appendix A – Borehole Logs And Explanatory Notes

BOREHOLE LOG

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BH NO. BH101

Project	Proposed Development	**Sheet**	1 of 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	23/06/2021
Position	Refer to Figure 2	**Date Completed**	24/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 23/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.06 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

Drilling					Sampling		Field Material Description					
METHOD	PENETRATION RESISTANCE	WATER	DEPTH (metres)	DEPTH RL	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	GROUP SYMBOL	SOIL/ROCK MATERIAL DESCRIPTION	MOISTURE CONDITION	CONSISTENCY REL. DENSITY	STRUCTURE AND ADDITIONAL OBSERVATIONS
DT			0	0.10 12.96				-	CONCRETE; 100 mm thick.	-	-	PAVEMENT
								-	FILL: Sandy CLAY; low plasticity, dark brown, with angular gravels, brick fragments.	-	-	FILL
				0.80 12.26				CI	Silty CLAY; medium plasticity, grey mottled orange.	M (>PL)	(VSt)	RESIDUAL SOIL
AD/T		GWNE	1 2 3									
				3.50 9.56					From 3.5 m, grading to extremely weathered sandstone, with ironstone bands.			
				3.80 9.26				-	SANDSTONE; fine to medium grained, grey, very low strength, distinctly weathered.			BEDROCK
	-		4	4.70 8.36					From 4.7 m, medium strength, slightly weathered.			
			5							-	-	
WB			6 7									
				7.70 5.36				-	SHALE; dark grey.			
			8 9							-	-	
			10									

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA NON-CORED BOREHOLE 1 E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:38 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

BOREHOLE LOG

BH NO. BH101

Project	Proposed Development	**Sheet**	2 of 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	23/06/2021
Position	Refer to Figure 2	**Date Completed**	24/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 23/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.06 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

Drilling					Sampling		Field Material Description					
METHOD	PENETRATION RESISTANCE	WATER	DEPTH (metres)	DEPTH RL	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	GROUP SYMBOL	SOIL/ROCK MATERIAL DESCRIPTION	MOISTURE CONDITION	CONSISTENCY REL. DENSITY	STRUCTURE AND ADDITIONAL OBSERVATIONS
WB	-		10					-	SHALE; dark grey.	-	-	BEDROCK
			11									
			12									
			13									
			14									
			15									
			16	16.55								
			17						Continued as Cored Borehole			
			18									
			19									
			20									

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA NON-CORED BOREHOLE 1 E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:38 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

CORED BOREHOLE LOG

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BH NO. BH101

Project	Proposed Development	**Sheet**	3 OF 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	23/06/2021
Position	Refer to Figure 2	**Date Completed**	24/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 23/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.06 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

Drilling						Field Material Description				Defect Information	
METHOD	WATER	TCR	RQD (SCR)	DEPTH (metres)	*DEPTH* RL	GRAPHIC LOG	ROCK / SOIL MATERIAL DESCRIPTION	WEATHERING	INFERRED STRENGTH $Is_{(50)}$ MPa (VL 0.1, L 0.3, M 1, H 3, VH 10, EH)	DEFECT DESCRIPTION & Additional Observations	Average Defect Spacing (mm) (30, 100, 300, 1000, 3000)
				10							
				11							
				12							
				13							
				14							
				15							
				16							
					16.55		Continuation from non-cored borehole				
NMLC	90-100% RETURN	100	94	17	-3.49		SHALE; dark grey, with pale grey laminations, medium bedded.	SW			
		67	25	18	*18.00* -4.94		From 18.0 m, thinly bedded.				
				19							
				20							

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

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CORED BOREHOLE LOG

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BH NO. BH101

Project	Proposed Development	**Sheet**	4 OF 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	23/06/2021
Position	Refer to Figure 2	**Date Completed**	24/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 23/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.06 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

Drilling						Field Material Description				Defect Information	
METHOD	WATER	TCR	RQD (SCR)	DEPTH (metres)	*DEPTH* RL	GRAPHIC LOG	ROCK / SOIL MATERIAL DESCRIPTION	WEATHERING	INFERRED STRENGTH $Is_{(50)}$ MPa (VL 0.1, L 0.3, M 1, H 3, VH 10, EH)	DEFECT DESCRIPTION & Additional Observations	Average Defect Spacing (mm) (30, 100, 300, 1000, 3000)
NMLC	90-100% RETURN	67	25	20			SHALE; dark grey, with pale grey laminations, medium bedded.	SW			
		100	24	21							
				22	*22.00* -8.94		Hole Terminated at 22.00 m Target Depth Reached.				
				23							
				24							
				25							
				26							
				27							
				28							
				29							
				30							

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA CORED BOREHOLE 1 E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:38 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

CORE PHOTOGRAPH OF BOREHOLE: BH101

Project	Proposed Development			**Depth Range**	16.55m to 22.0m BEGL		
Location	28 Elizabeth Street, Liverpool NSW			**Contractor**	BG Drilling Pty Ltd		
Position	See Figure 2	**Surface RL**	≈ 13.06m	**Drill Rig**	Hanjin D&B 8D		
Job No.	E24175.G04	**Inclination**	-90°	**Logged**	KX	**Date**	23 / 06 / 2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Box**	1-2 of 2	**Checked**	MT	**Date**	22 / 07 / 2021

BOREHOLE LOG

BH NO. BH102M

Project	Proposed Development	**Sheet**	1 of 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	24/06/2021
Position	Refer to Figure 2	**Date Completed**	24/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 24/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.24 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

Drilling					Sampling		Field Material Description					
METHOD	PENETRATION RESISTANCE	WATER	DEPTH (metres)	DEPTH RL	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	GROUP SYMBOL	SOIL/ROCK MATERIAL DESCRIPTION	MOISTURE CONDITION	CONSISTENCY REL. DENSITY	STRUCTURE AND ADDITIONAL OBSERVATIONS
DT			0	13.21				-	ASPHALT; 30 mm thick.	-	-	PAVEMENT
								-	FILL: Gravelly SAND; fine to coarse grained, dark grey, fine to coarse, sub-angular to sub-rounded gravels, with clay.	M	-	FILL
				0.60								
				12.64				CI-CH	Silty CLAY; medium to high plasticity, pale brown-grey mottled red-brown.			RESIDUAL SOIL
			1									
			2							M (>PL)	(St)	
			3									
				3.40								
AD/T		GWNE		9.84					From 3.4 m, with fine to coarse, sub-angular to sub-rounded ironstone gravels.			
				3.60								
				9.64					From 3.6 m, grading to extremely weathered material.			
			4									
				4.50								
				8.74				-	SANDSTONE; fine to medium grained, pale grey-brown, very low strength, distinctly weathered.			BEDROCK
	-		5									
			6									
										-	-	
WB			7									
			8									
				9.00								
			9	4.24				-	SHALE; dark grey.			
										-	-	
			10									

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

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BOREHOLE LOG

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BH NO. BH102M

Project	Proposed Development	**Sheet**	2 of 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	24/06/2021
Position	Refer to Figure 2	**Date Completed**	24/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 24/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.24 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

Drilling					Sampling		Field Material Description					
METHOD	PENETRATION RESISTANCE	WATER	DEPTH (metres)	DEPTH RL	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	GROUP SYMBOL	SOIL/ROCK MATERIAL DESCRIPTION	MOISTURE CONDITION	CONSISTENCY REL. DENSITY	STRUCTURE AND ADDITIONAL OBSERVATIONS
WB	-		10					-	SHALE; dark grey.	-	-	BEDROCK
			11									
			12									
			13									
			14									
			15									
			16	16.60								
									Continued as Cored Borehole			
			17									
			18									
			19									
			20									

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA NON-CORED BOREHOLE 1 E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:38 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

CORED BOREHOLE LOG

BH NO. BH102M

Project	Proposed Development	**Sheet**	3 OF 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	24/06/2021
Position	Refer to Figure 2	**Date Completed**	24/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 24/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.24 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

Drilling						Field Material Description				Defect Information	
METHOD	WATER	TCR	RQD (SCR)	DEPTH (metres)	*DEPTH* RL	GRAPHIC LOG	ROCK / SOIL MATERIAL DESCRIPTION	WEATHERING	INFERRED STRENGTH $Is_{(50)}$ MPa (VL 0.1, L 0.3, M 1, H 3, VH 10, EH)	DEFECT DESCRIPTION & Additional Observations	Average Defect Spacing (mm) (30, 100, 300, 1000, 3000)
				10							
				11							
				12							
				13							
				14							
				15							
				16							
					16.60		Continuation from non-cored borehole				
NMLC	90-100% RETURN	100	95	17	-3.36		SHALE; dark grey, with pale grey laminations, medium bedded.	FR			
				18							
		100	79	19							
				20							

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA CORED BOREHOLE 1 E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:38 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

CORED BOREHOLE LOG

BH NO. BH102M

Project	Proposed Development	**Sheet**	4 OF 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	24/06/2021
Position	Refer to Figure 2	**Date Completed**	24/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 24/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.24 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

Drilling						Field Material Description				Defect Information	
METHOD	WATER	TCR	RQD (SCR)	DEPTH (metres)	*DEPTH* RL	GRAPHIC LOG	ROCK / SOIL MATERIAL DESCRIPTION	WEATHERING	INFERRED STRENGTH $Is_{(50)}$ MPa (VL 0.1, L 0.3, M 1, H 3, VH 10, EH)	DEFECT DESCRIPTION & Additional Observations	Average Defect Spacing (mm) (30, 100, 300, 1000, 3000)
NMLC	90-100% RETURN	100	79	20			SHALE; dark grey, with pale grey laminations, medium bedded.	FR			
		100	97	21							
				22							
				23	*23.00* -9.76		Hole Terminated at 23.00 m Target Depth Reached.				
				24							
				25							
				26							
				27							
				28							
				29							
				30							

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA CORED BOREHOLE 1 E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:38 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

MONITORING WELL LOG

MW NO. BH102M

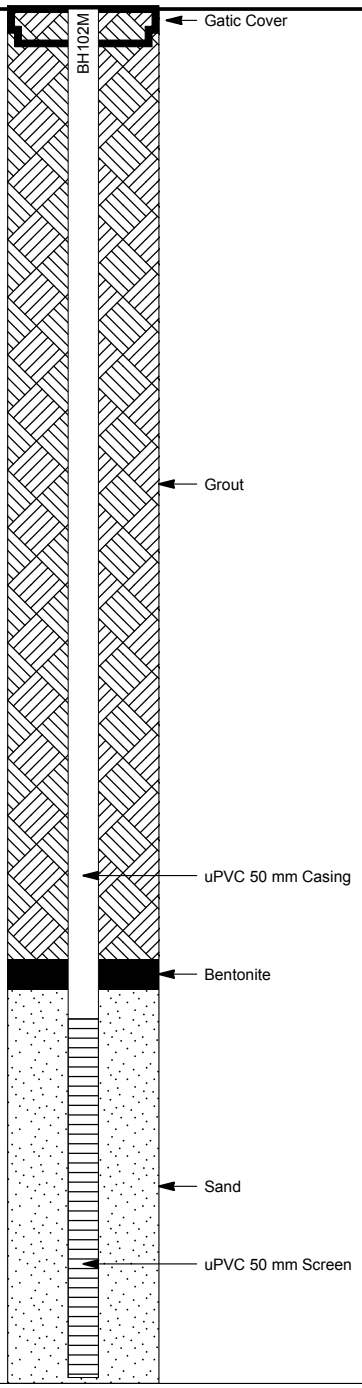
Project	Proposed Development	**Sheet**	1 of 2
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	24/06/2021
Position	Refer to Figure 2	**Date Completed**	24/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 24/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.24 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

METHOD	WATER	DEPTH (m)	RL (m AHD)	GRAPHIC LOG	SOIL/ROCK MATERIAL DESCRIPTION
DT / AD/T	GWNE	0	12		ASPHALT; 30 mm thick.
		0.03	12		FILL: Gravelly SAND; fine to coarse grained, dark grey, fine to coarse, sub-angular to sub-rounded gravels, with clay.
		0.5	11.5		Silty CLAY; medium to high plasticity, pale brown-grey mottled red-brown.
		3.4	9.6		From 3.4 m, with fine to coarse, sub-angular to sub-rounded ironstone gravels.
		3.6	9.4		From 3.6 m, grading to extremely weathered material.
		4.0	9.2		SANDSTONE; fine to medium grained, pale grey-brown, very low strength, distinctly weathered.
WB		8.0	5.2		SHALE; dark grey.
NMLC	90-100% RETURN	16.90	-3.70		SHALE; dark grey, with pale grey laminations, medium bedded.
		22.90	-9.66		Hole Terminated at 23.00 m Target Depth Reached.

PIEZOMETER CONSTRUCTION DETAILS

ID	Type	Stick Up & RL	Tip Depth & RL	Installation Date Static Water Level
BH102M	Standpipe		22.90 m -9.66 m	

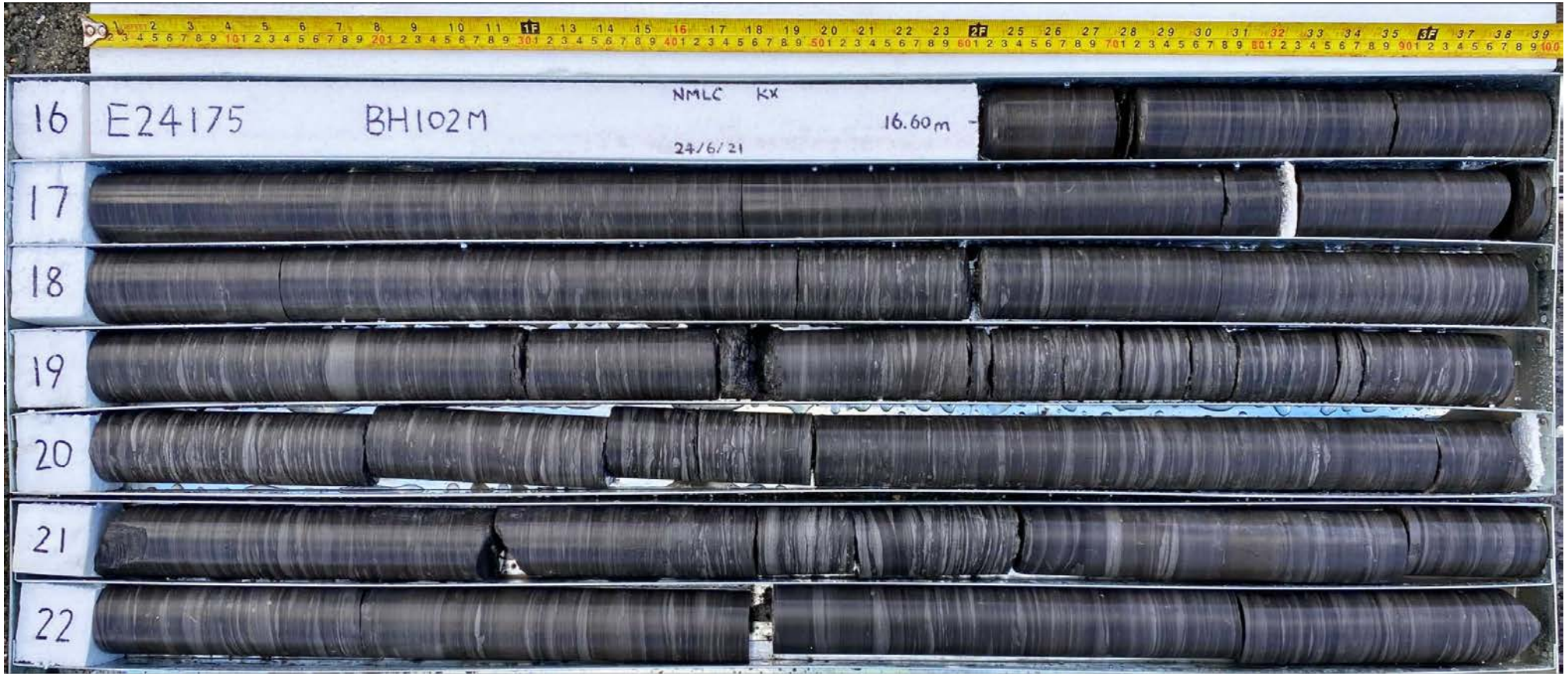


This well log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA PIEZOMETER INSTALLATION LOG E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:40 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

CORE PHOTOGRAPH OF BOREHOLE: BH102M

Project	Proposed Development			**Depth Range**	16.6m to 23.0m BEGL		
Location	28 Elizabeth Street, Liverpool NSW			**Contractor**	BG Drilling Pty Ltd		
Position	See Figure 2	**Surface RL**	≈ 13.24m	**Drill Rig**	Hanjin D&B 8D		
Job No.	E24175.G04	**Inclination**	-90°	**Logged**	KX	**Date**	24 / 06 / 2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Box**	1-2 of 2	**Checked**	MT	**Date**	22 / 07 / 2021



BOREHOLE LOG

BH NO. BH103

Project	Proposed Development	**Sheet**	1 of 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	24/06/2021
Position	Refer to Figure 2	**Date Completed**	25/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 24/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.08 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

Drilling					Sampling		Field Material Description					
METHOD	PENETRATION RESISTANCE	WATER	DEPTH (metres)	DEPTH RL	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	GROUP SYMBOL	SOIL/ROCK MATERIAL DESCRIPTION	MOISTURE CONDITION	CONSISTENCY REL. DENSITY	STRUCTURE AND ADDITIONAL OBSERVATIONS
AD/T	-	GWNE	0	13.08				-	FILL: Silty SAND; fine grained, dark brown, with gravels and brick fragments, with clay.	M	-	FILL
				1.00								
			1	12.08				CH	Silty CLAY; high plasticity, grey mottled red.	M (>PL)	(St)	RESIDUAL SOIL
			2									
			3									
				3.50								
				9.58					From 3.5 m, grading to extremely weathered material.			
			4	*4.00*								
				9.08				-	SANDSTONE; pale grey, very low strength, distinctly weathered, with ironstaining.			BEDROCK
WB			5							-	-	
			6									
			7									
				7.70								
				5.38				-	SHALE; dark grey.			
			8									
			9							-	-	
			10									

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA NON-CORED BOREHOLE 1 E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:38 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

BOREHOLE LOG

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BH NO. BH103

Project	Proposed Development	**Sheet**	2 of 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	24/06/2021
Position	Refer to Figure 2	**Date Completed**	25/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 24/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.08 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

Drilling					Sampling		Field Material Description					
METHOD	PENETRATION RESISTANCE	WATER	DEPTH (metres)	*DEPTH* RL	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	GROUP SYMBOL	SOIL/ROCK MATERIAL DESCRIPTION	MOISTURE CONDITION	CONSISTENCY REL. DENSITY	STRUCTURE AND ADDITIONAL OBSERVATIONS
WB	-		10					-	SHALE; dark grey.	-	-	BEDROCK
			11									
			12									
			13									
			14									
			15									
			16	*16.60*								
									Continued as Cored Borehole			
			17									
			18									
			19									
			20									

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA NON-CORED BOREHOLE 1 E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:38 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

CORED BOREHOLE LOG

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BH NO. BH103

Project	Proposed Development	**Sheet**	3 OF 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	24/06/2021
Position	Refer to Figure 2	**Date Completed**	25/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 24/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.08 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

Drilling						Field Material Description				Defect Information	
METHOD	WATER	TCR	RQD (SCR)	DEPTH (metres)	*DEPTH* RL	GRAPHIC LOG	ROCK / SOIL MATERIAL DESCRIPTION	WEATHERING	INFERRED STRENGTH $Is_{(50)}$ MPa (VL 0.1, L 0.3, M 1, H 3, VH 10, EH)	DEFECT DESCRIPTION & Additional Observations	Average Defect Spacing (mm) (30, 100, 300, 1000, 3000)
				10							
				11							
				12							
				13							
				14							
				15							
				16							
					16.60		Continuation from non-cored borehole				
NMLC	90-100% RETURN	100	7	17	-3.52		SHALE; dark grey, with pale grey laminations, very thinly bedded.	SW			
					17.61		From 17.61 m, thinly bedded.				
				18	-4.53					18.19-18.21: XWS, 20 mm	
		100	55	19							
				20							

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA CORED BOREHOLE 1 E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:38 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

CORED BOREHOLE LOG

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BH NO. BH103

Project	Proposed Development	**Sheet**	4 OF 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	24/06/2021
Position	Refer to Figure 2	**Date Completed**	25/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 24/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.08 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

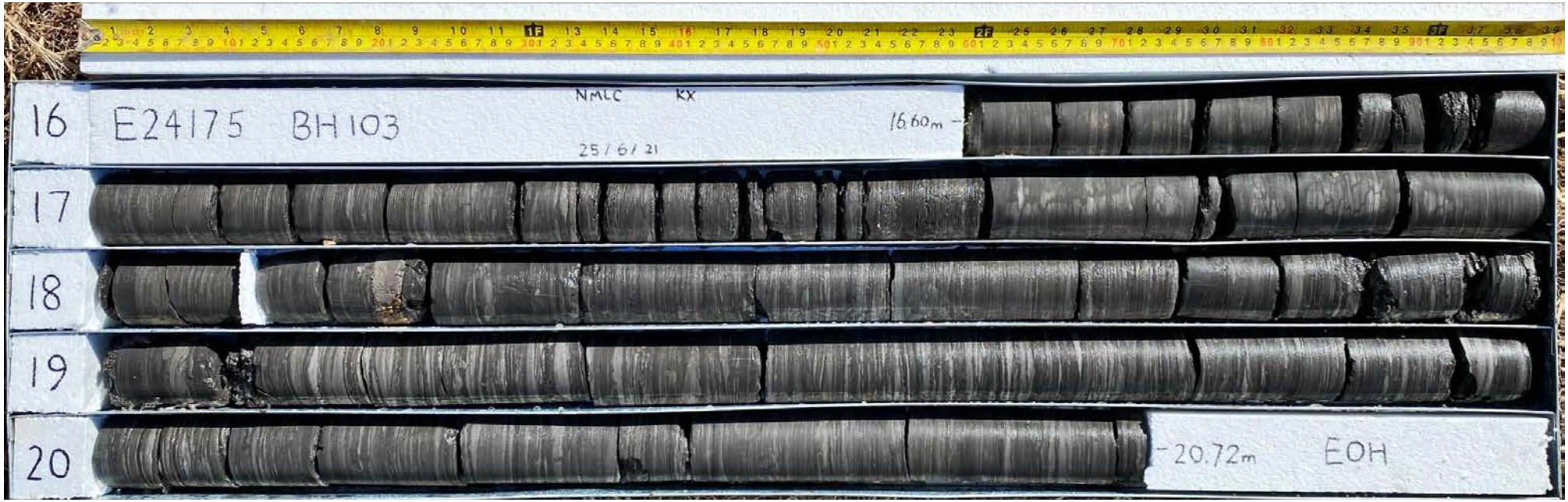
Drilling						Field Material Description				Defect Information	
METHOD	WATER	TCR	RQD (SCR)	DEPTH (metres)	*DEPTH* RL	GRAPHIC LOG	ROCK / SOIL MATERIAL DESCRIPTION	WEATHERING	INFERRED STRENGTH $Is_{(50)}$ MPa (VL 0.1, L 0.3, M 1, H 3, VH 10, EH)	DEFECT DESCRIPTION & Additional Observations	Average Defect Spacing (mm) (30, 100, 300, 1000, 3000)
NMLC		100	55	20			SHALE; dark grey, with pale grey laminations, very thinly bedded.	SW			
					20.72 -7.64		Hole Terminated at 20.72 m Target Depth Reached.				
				21							
				22							
				23							
				24							
				25							
				26							
				27							
				28							
				29							
				30							

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA CORED BOREHOLE 1 E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:38 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

CORE PHOTOGRAPH OF BOREHOLE: BH103

Project	Proposed Development			**Depth Range**	16.6m to 20.72m BEGL		
Location	28 Elizabeth Street, Liverpool NSW			**Contractor**	BG Drilling Pty Ltd		
Position	See Figure 2	**Surface RL**	≈ 13.08m	**Drill Rig**	Hanjin D&B 8D		
Job No.	E24175.G04	**Inclination**	-90°	**Logged**	KX	**Date**	24 / 06 / 2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Box**	1 of 1	**Checked**	MT	**Date**	22 / 07 / 2021



BOREHOLE LOG

BH NO. BH104

Project	Proposed Development	**Sheet**	1 of 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	25/06/2021
Position	Refer to Figure 2	**Date Completed**	25/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 25/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.76 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

Drilling					Sampling		Field Material Description					
METHOD	PENETRATION RESISTANCE	WATER	DEPTH (metres)	DEPTH RL	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	GROUP SYMBOL	SOIL/ROCK MATERIAL DESCRIPTION	MOISTURE CONDITION	CONSISTENCY REL. DENSITY	STRUCTURE AND ADDITIONAL OBSERVATIONS
AD/T	-	GWNE	0	13.76				-	FILL: gravelly SAND; fine to coarse grained, dark grey, fine to coarse, sub-angular to sub-rounded gravels.	M	-	FILL
				0.60								
				13.16				Cl	Silty CLAY; medium plasticity, brown mottled pale grey.			RESIDUAL SOIL
			1									
				2.00								
			2	11.76					From 2.0 m, pale grey mottled red-brown, with fine, sub-angular to sub-rounded ironstone gravels, fine grained sand.			
			3							M (>PL)	(St)	
				3.70								
				10.06					From 3.7 m, pale grey-brown.			
			4									
			5									
				6.00								
			6	7.76					From 6.0 m, grading to extremely weathered material.			
				6.50								
				7.26				-	SANDSTONE; brown, very low strength, distinctly weathered, with shale.	-	-	BEDROCK
			7									
				7.70								
WB				6.06				-	SHALE; dark grey.			
			8									
			9							-	-	
			10									

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA NON-CORED BOREHOLE 1 E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:38 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

BOREHOLE LOG

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BH NO. BH104

Project	Proposed Development	**Sheet**	2 of 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	25/06/2021
Position	Refer to Figure 2	**Date Completed**	25/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 25/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.76 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

Drilling					Sampling		Field Material Description					
METHOD	PENETRATION RESISTANCE	WATER	DEPTH (metres)	DEPTH RL	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	GROUP SYMBOL	SOIL/ROCK MATERIAL DESCRIPTION	MOISTURE CONDITION	CONSISTENCY REL. DENSITY	STRUCTURE AND ADDITIONAL OBSERVATIONS
WB	-		10					-	SHALE; dark grey.	-	-	BEDROCK
			11									
			12									
			13									
			14									
			15									
			16	16.60								
									Continued as Cored Borehole			
			17									
			18									
			19									
			20									

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA NON-CORED BOREHOLE 1 E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:38 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

CORED BOREHOLE LOG

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BH NO. BH104

Project	Proposed Development	**Sheet**	3 OF 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	25/06/2021
Position	Refer to Figure 2	**Date Completed**	25/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 25/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.76 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

Drilling						Field Material Description				Defect Information	
METHOD	WATER	TCR	RQD (SCR)	DEPTH (metres)	*DEPTH* RL	GRAPHIC LOG	ROCK / SOIL MATERIAL DESCRIPTION	WEATHERING	INFERRED STRENGTH $Is_{(50)}$ MPa (VL 0.1, L 0.3, M 1, H 3, VH 10, EH)	DEFECT DESCRIPTION & Additional Observations	Average Defect Spacing (mm) (30, 100, 300, 1000, 3000)
				10							
				11							
				12							
				13							
				14							
				15							
				16							
					16.60		Continuation from non-cored borehole				
NMLC	90-100% RETURN	100	96	17	-2.84		SHALE; dark grey, with pale grey laminations, medium bedded.	FR			
				18	*18.17* -4.41		From 18.17 m, thinly bedded.				
		100	46	19						19.27-19.34: CS, 70 mm	
				20	*20.00*						

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA CORED BOREHOLE 1 E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:39 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

CORED BOREHOLE LOG

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BH NO. BH104

Project	Proposed Development	**Sheet**	4 OF 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	25/06/2021
Position	Refer to Figure 2	**Date Completed**	25/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 25/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.76 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

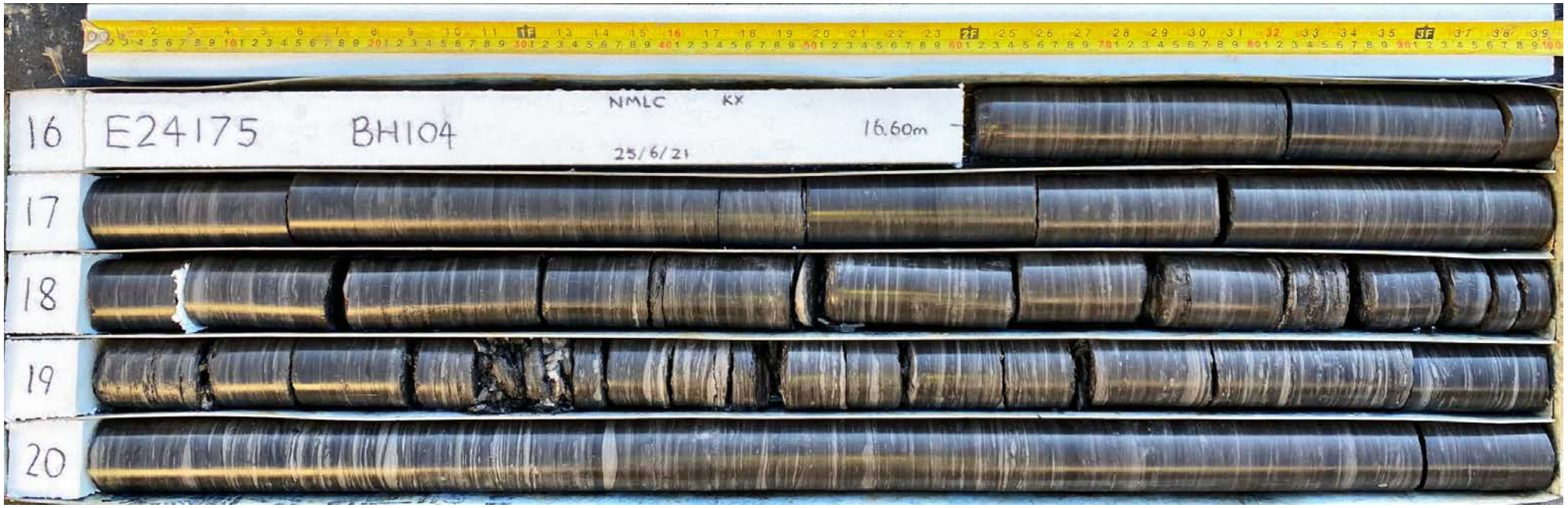
Drilling						Field Material Description				Defect Information	
METHOD	WATER	TCR	RQD (SCR)	DEPTH (metres)	*DEPTH* RL	GRAPHIC LOG	ROCK / SOIL MATERIAL DESCRIPTION	WEATHERING	INFERRED STRENGTH $Is_{(50)}$ MPa (VL 0.1, L 0.3, M 1, H 3, VH 10, EH)	DEFECT DESCRIPTION & Additional Observations	Average Defect Spacing (mm) (30, 100, 300, 1000, 3000)
NMLC	90-100% RETURN	100	46	20	-6.24		SHALE; dark grey, with pale grey laminations, medium bedded. From 20.0 m, thickly bedded.	FR			
				21	*21.00* -7.24		Hole Terminated at 21.00 m Target Depth Reached.				
				22							
				23							
				24							
				25							
				26							
				27							
				28							
				29							
				30							

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA CORED BOREHOLE 1 E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:39 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

CORE PHOTOGRAPH OF BOREHOLE: BH104

Project	Proposed Development			**Depth Range**	16.6m to 21.0m BEGL		
Location	28 Elizabeth Street, Liverpool NSW			**Contractor**	BG Drilling Pty Ltd		
Position	See Figure 2	**Surface RL**	≈ 13.76m	**Drill Rig**	Hanjin D&B 8D		
Job No.	E24175.G04	**Inclination**	-90°	**Logged**	KX	**Date**	25 / 06 / 2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Box**	1 of 1	**Checked**	MT	**Date**	22 / 07 / 2021



BOREHOLE LOG

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BH NO. BH105

Project	Proposed Development	**Sheet**	1 of 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	28/06/2021
Position	Refer to Figure 2	**Date Completed**	28/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 28/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.96 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

Drilling					Sampling		Field Material Description					
METHOD	PENETRATION RESISTANCE	WATER	DEPTH (metres)	DEPTH RL	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	GROUP SYMBOL	SOIL/ROCK MATERIAL DESCRIPTION	MOISTURE CONDITION	CONSISTENCY REL. DENSITY	STRUCTURE AND ADDITIONAL OBSERVATIONS
DT			0	0.12				-	CONCRETE; 120 mm thick.	-	-	PAVEMENT
				13.84				-	FILL: Silty SAND; medium to coarse grained, orange-brown, trace ash.	M	-	FILL
				0.60								
				13.36				-	FILL: Silty CLAY; low plasticity, dark brown, with sand, trace gravels.	M		
				0.90								
			1	13.06				CI	Silty CLAY; medium plasticity, grey mottled red.			RESIDUAL SOIL
AD/T		GWNE	2							M (>PL)	(St)	
			3	3.20								
				10.76					From 3.2 m, grey, with ironstone bands, grading to extremely weathered material.			
			4	4.80								
	-			9.16				-	SHALE; dark grey, very low strength, distinctly weathered.			BEDROCK
			5	5.50								
				8.46					From 5.5 m, dark grey.			
WB			6							-	-	
			7									
			8									
			9									
			10									

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA NON-CORED BOREHOLE 1 E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:38 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

BOREHOLE LOG

BH NO. BH105

Project	Proposed Development	**Sheet**	2 of 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	28/06/2021
Position	Refer to Figure 2	**Date Completed**	28/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 28/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.96 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

Drilling					Sampling		Field Material Description					
METHOD	PENETRATION RESISTANCE	WATER	DEPTH (metres)	DEPTH RL	SAMPLE OR FIELD TEST	RECOVERED	GRAPHIC LOG	GROUP SYMBOL	SOIL/ROCK MATERIAL DESCRIPTION	MOISTURE CONDITION	CONSISTENCY REL. DENSITY	STRUCTURE AND ADDITIONAL OBSERVATIONS
WB	-		10					-	SHALE; dark grey.	-	-	BEDROCK
			11									
			12									
			13									
			14									
			15									
			16	16.50								
									Continued as Cored Borehole			
			17									
			18									
			19									
			20									

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA NON-CORED BOREHOLE 1 E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:38 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

CORED BOREHOLE LOG

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BH NO. BH105

Project	Proposed Development	**Sheet**	3 OF 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	28/06/2021
Position	Refer to Figure 2	**Date Completed**	28/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 28/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.96 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

Drilling						Field Material Description				Defect Information	
METHOD	WATER	TCR	RQD (SCR)	DEPTH (metres)	*DEPTH* RL	GRAPHIC LOG	ROCK / SOIL MATERIAL DESCRIPTION	WEATHERING	INFERRED STRENGTH $Is_{(50)}$ MPa (VL 0.1, L 0.3, M 1, H 3, VH 10, EH)	DEFECT DESCRIPTION & Additional Observations	Average Defect Spacing (mm) (30, 100, 300, 1000, 3000)
				10							
				11							
				12							
				13							
				14							
				15							
				16							
					16.50		Continuation from non-cored borehole				
NMLC	90-100% RETURN	100	71	17	-2.54		SHALE; dark grey, with pale grey laminations, medium bedded.	FR			
				18						18.33-18.36: CS, 30 mm 18.43: JT, 30°, CN, PR, RF	
				19						18.77: JT, 40 - 50°, CN, PR, RF	
		100	62	20						19.62-19.77: CZ, 150 mm	

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA CORED BOREHOLE 1 E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:39 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

CORED BOREHOLE LOG

eiaustralia
Practical Solutions for Built Environments

BH NO. BH105

Project	Proposed Development	**Sheet**	4 OF 4
Location	28 Elizabeth Street, Liverpool NSW	**Date Started**	28/06/2021
Position	Refer to Figure 2	**Date Completed**	28/06/2021
Job No.	E24175.G04	**Logged By** KX	**Date** 28/06/2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Reviewed By** MT	**Date** 22/07/2021

Drilling Contactor	BG Drilling Pty Ltd	**Surface RL**	≈13.96 m AHD
Drill Rig	Hanjin DB 8D	**Inclination**	-90°

Drilling						Field Material Description				Defect Information	
METHOD	WATER	TCR	RQD (SCR)	DEPTH (metres)	*DEPTH* RL	GRAPHIC LOG	ROCK / SOIL MATERIAL DESCRIPTION	WEATHERING	INFERRED STRENGTH $Is_{(50)}$ MPa (VL 0.1, L 0.3, M 1, H 3, VH 10, EH)	DEFECT DESCRIPTION & Additional Observations	Average Defect Spacing (mm) (30, 100, 300, 1000, 3000)
NMLC	90-100% RETURN	100	62	20			SHALE; dark grey, with pale grey laminations, medium bedded.	FR			
				21							
				22							
					22.33 -8.37		Hole Terminated at 22.33 m Target Depth Reached.				
				23							
				24							
				25							
				26							
				27							
				28							
				29							
				30							

This borehole log should be read in conjunction with EI Australia's accompanying standard notes.

EIA 2.00.3 LIB.GLB Log EIA CORED BOREHOLE 1 E24175.G04 BOREHOLE LOGS.GPJ <<DrawingFile>> 23/07/2021 11:39 10.0.000 Datgel Lab and In Situ Tool - DGD | Lib: EIA 2.00.3 2017-11-21 Prj: EIA 2.00.1 2017-09-26

CORE PHOTOGRAPH OF BOREHOLE: BH105

Project	Proposed Development			**Depth Range**	16.5m to 22.33m BEGL		
Location	28 Elizabeth Street, Liverpool NSW			**Contractor**	BG Drilling Pty Ltd		
Position	See Figure 2	**Surface RL**	≈ 13.96m	**Drill Rig**	Hanjin D&B 8D		
Job No.	E24175.G04	**Inclination**	-90°	**Logged**	KX	**Date**	28 / 06 / 2021
Client	Altis Bulky Retail Pty Ltd as trustee for Altis ARET Sub Trust 20	**Box**	1-2 of 2	**Checked**	MT	**Date**	22 / 07 / 2021

EXPLANATION OF NOTES, ABBREVIATIONS & TERMS USED ON BOREHOLE AND TEST PIT LOGS

DRILLING/EXCAVATION METHOD

HA	Hand Auger	**ADH**	Hollow Auger	**NQ**	Diamond Core - 47 mm
DT	Diatube Coring	**RT**	Rotary Tricone bit	**NMLC**	Diamond Core - 52 mm
NDD	Non-destructive digging	**RAB**	Rotary Air Blast	**HQ**	Diamond Core - 63 mm
AD*	Auger Drilling	**RC**	Reverse Circulation	**HMLC**	Diamond Core - 63 mm
***V**	V-Bit	**PT**	Push Tube	**EX**	Tracked Hydraulic Excavator
***T**	TC-Bit, e.g. AD/T	**WB**	Washbore	**HAND**	Excavated by Hand Methods

PENETRATION RESISTANCE

L	**Low Resistance**	Rapid penetration/ excavation possible with little effort from equipment used.
M	**Medium Resistance**	Penetration/ excavation possible at an acceptable rate with moderate effort from equipment used.
H	**High Resistance**	Penetration/ excavation is possible but at a slow rate and requires significant effort from equipment used.
R	**Refusal/Practical Refusal**	No further progress possible without risk of damage or unacceptable wear to equipment used.

These assessments are subjective and are dependent on many factors, including equipment power and weight, condition of excavation or drilling tools and experience of the operator.

WATER

Standing Water Level — **Partial water loss**

Water Seepage — **Complete Water Loss**

GWNO	GROUNDWATER NOT OBSERVED - Observation of groundwater, whether present or not, was not possible due to drilling water, surface seepage or cave-in of the borehole/ test pit.
GWNE	GROUNDWATER NOT ENCOUNTERED - Borehole/ test pit was dry soon after excavation. However, groundwater could be present in less permeable strata. Inflow may have been observed had the borehole/ test pit been left open for a longer period.

SAMPLING AND TESTING

SPT	Standard Penetration Test to AS1289.6.3.1-2004
4,7,11 N=18	4,7,11 = Blows per 150mm. N = Blows per 300mm penetration following a 150mm seating drive
30/80mm	Where practical refusal occurs, the blows and penetration for that interval are reported, N is not reported
RW	Penetration occurred under the rod weight only, N<1
HW	Penetration occurred under the hammer and rod weight only, N<1
HB	Hammer double bouncing on anvil, N is not reported
Sampling	
DS	Disturbed Sample
ES	Sample for environmental testing
BDS	Bulk disturbed Sample
GS	Gas Sample
WS	Water Sample
U50	Thin walled tube sample - number indicates nominal sample diameter in millimetres
Testing	
FP	Field Permeability test over section noted
FVS	Field Vane Shear test expressed as uncorrected shear strength (sv= peak value, sr= residual value)
PID	Photoionisation Detector reading in ppm
PM	Pressuremeter test over section noted
PP	Pocket Penetrometer test expressed as instrument reading in kPa
WPT	Water Pressure tests
DCP	Dynamic Cone Penetrometer test
CPT	Static Cone Penetration test
CPTu	Static Cone Penetration test with pore pressure (u) measurement

GEOLOGICAL BOUNDARIES

———— = Observed Boundary (position known)

– – – – – – – – – – = Observed Boundary (position approximate)

– –?– –?– –?– – = Boundary (interpreted or inferred)

ROCK CORE RECOVERY

TCR=Total Core Recovery (%)

$$= \frac{\textit{Length of core recovered}}{\textit{Length of core run}} \times 100$$

RQD = Rock Quality Designation (%)

$$= \frac{\sum \textit{Axial lengths of core} > 100mm}{\textit{Length of core run}} \times 100$$

METHOD OF SOIL DESCRIPTION USED ON BOREHOLE AND TEST PIT LOGS

	FILL		ORGANIC SOILS (OL, OH or Pt)		CLAY (CL, CI or CH)
	COUBLES or BOULDERS		SILT (ML or MH)		SAND (SP or SW)
	GRAVEL (GP or GW)	Combinations of these basic symbols may be used to indicate mixed materials such as sandy clay			

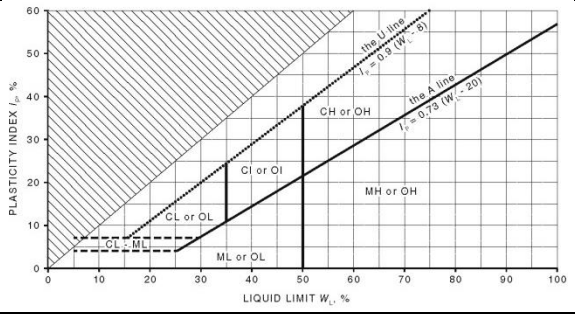
CLASSIFICATION AND INFERRED STRATIGRAPHY

Soil is broadly classified and described in Borehole and Test Pit Logs using the preferred method given in AS 1726:2017, Section 6.1 – Soil description and classification.

PARTICLE SIZE CHARACTERISTICS

Fraction	Components	Sub Division	Size mm
Oversize	BOULDERS		>200
	COBBLES		63 to 200
Coarse grained soil	GRAVEL	Coarse	19 to 63
		Medium	6.7 to 19
		Fine	2.36 to 6.7
	SAND	Coarse	0.6 to 2.36
		Medium	0.21 to 0.6
		Fine	0.075 to 0.21
Fine grained soil	SILT		0.002 to 0.075
	CLAY		<0.002

PLASTICITY PROPERTIES



GROUP SYMBOLS

Major Divisions		Symbol	Description
COARSE GRAINED SOILS More than 65% of soil excluding oversize fraction is greater than 0.075mm	GRAVEL More than 50% of coarse fraction is >2.36mm	GW	Well graded gravel and gravel-sand mixtures, little or no fines, no dry strength.
		GP	Poorly graded gravel and gravel-sand mixtures, little or no fines, no dry strength.
		GM	Silty gravel, gravel-sand-silt mixtures, zero to medium dry strength.
		GC	Clayey gravel, gravel-sand-clay mixtures, medium to high dry strength.
	SAND More than 50% of coarse fraction is <2.36 mm	SW	Well graded sand and gravelly sand, little or no fines, no dry strength.
		SP	Poorly graded sand and gravelly sand, little or no fines, no dry strength.
		SM	Silty sand, sand-silt mixtures, zero to medium dry strength.
		SC	Clayey sand, sandy-clay mixtures, medium to high dry strength.
FINE GRAINED SOILS More than 35% of soil excluding oversized fraction is less than 0.075mm	Liquid Limit less < 50%	ML	Inorganic silts of low plasticity, very fine sands, rock flour, silty or clayey fine sands, zero to medium dry strength.
		CL, CI	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, medium to high dry strength.
		OL	Organic silts and organic silty clays of low plasticity, low to medium dry strength.
	Liquid Limit > than 50%	MH	Inorganic silts of high plasticity, high to very high dry strength.
		CH	Inorganic clays of high plasticity, high to very high dry strength.
		OH	Organic clays of medium to high plasticity, medium to high dry strength.
Highly Organic soil		PT	Peat muck and other highly organic soils.

MOISTURE CONDITION

Symbol	Term	Description
D	Dry	Non- cohesive and free-running.
M	Moist	Soils feel cool, darkened in colour. Soil tends to stick together.
W	Wet	Soils feel cool, darkened in colour. Soil tends to stick together, free water forms when handling.

Moisture content of cohesive soils shall be described in relation to plastic limit (PL) or liquid limit (LL) for soils with higher moisture content as follows: Moist, dry of plastic limit (w < PL); Moist, near plastic limit (w ≈ PL); Moist, wet of plastic limit (w < PL); Wet, near liquid limit (w ≈ LL), Wet, wet of liquid limit (w > LL),

CONSISTENCY

Symbol	Term	Undrained Shear Strength (kPa)	SPT "N" #
VS	Very Soft	≤ 12	≤ 2
S	Soft	>12 to ≤ 25	>2 to ≤ 4
F	Firm	>25 to ≤ 50	>4 to 8
St	Stiff	>50 to ≤ 100	>8 to 15
VSt	Very Stiff	>100 to ≤ 200	>15 to 30
H	Hard	>200	>30
Fr	Friable	-	

DENSITY

Symbol	Term	Density Index %	SPT "N" #
VL	Very Loose	≤ 15	0 to 4
L	Loose	>15 to ≤ 35	4 to 10
MD	Medium Dense	>35 to ≤ 65	10 to 30
D	Dense	>65 to ≤ 85	30 to 50
VD	Very Dense	>85	Above 50

In the absence of test results, consistency and density may be assessed from correlations with the observed behaviour of the material.
SPT correlations are not stated in AS1726:2017, and may be subject to corrections for overburden pressure, moisture content of the soil, and equipment type.

MINOR COMPONENTS

Term	Assessment Guide	Proportion by Mass
Add 'Trace'	Presence just detectable by feel or eye but soil properties little or no different to general properties of primary component	Coarse grained soils: ≤ 5% Fine grained soil: ≤ 15%
Add 'With'	Presence easily detectable by feel or eye but soil properties little or no different to general properties of primary component	Coarse grained soils: 5 - 12% Fine grained soil: 15 - 30%
Prefix soil name	Presence easily detectable by feel or eye in conjunction with the general properties of primary component	Coarse grained soils: >12% Fine grained soil: >30%

TERMS FOR ROCK MATERIAL STRENGTH AND WEATHERING

CLASSIFICATION AND INFERRED STRATIGRAPHY

Rock is broadly classified and described in Borehole and Test Pit Logs using the preferred method given in AS1726 – 2017, Section 6.2 – Rock identification, description and classification.

ROCK MATERIAL STRENGTH CLASSIFICATION

Symbol	Term	Point Load Index, $Is_{(50)}$ (MPa) #	Field Guide
VL	Very Low	0.03 to 0.1	Material crumbles under firm blows with sharp end of pick; can be peeled with knife; too hard to cut a triaxial sample by hand. Pieces up to 30 mm can be broken by finger pressure.
L	Low	0.1 to 0.3	Easily scored with a knife; indentations 1 mm to 3 mm show in the specimen with firm blows of pick point; has dull sound under hammer. A piece of core 150 mm long by 50 mm diameter may be broken by hand. Sharp edges of core may be friable and break during handling.
M	Medium	0.3 to 1	Readily scored with a knife; a piece of core 150 mm long by 50 mm diameter can be broken by hand with difficulty.
H	High	1 to 3	A piece of core 150 mm long by 50 mm diameter cannot be broken by hand but can be broken with pick with a single firm blow; rock rings under hammer.
VH	Very High	3 to 10	Hand specimen breaks with pick after more than one blow; rock rings under hammer.
EH	Extremely High	>10	Specimen requires many blows with geological pick to break through intact material; rock rings under hammer.

 # Rock Strength Test Results

▼ Point Load Strength Index, $Is_{(50)}$, Axial test (MPa)

● Point Load Strength Index, $Is_{(50)}$, Diametral test (MPa)

Relationship between rock strength test result ($Is_{(50)}$) and unconfined compressive strength (UCS) will vary with rock type and strength, and should be determined on a site-specific basis. However UCS is typically 20 x $Is_{(50)}$.

ROCK MATERIAL WEATHERING CLASSIFICATION

Symbol		Term	Field Guide
RS		Residual Soil	Soil developed on extremely weathered rock; the mass structure and substance fabric are no longer evident; there is a large change in volume but the soil has not been significantly transported.
XW		Extremely Weathered	Rock is weathered to such an extent that it has soil properties - i.e. it either disintegrates or can be remoulded, in water.
DW	HW	Distinctly Weathered	Rock strength usually changed by weathering. The rock may be highly discoloured, usually by iron staining. Porosity may be increased by leaching, or may be decreased due to deposition of weathering products in pores. In some environments it is convenient to subdivide into Highly Weathered and Moderately Weathered, with the degree of alteration typically less for MW.
	MW		
SW		Slightly Weathered	Rock slightly discoloured but shows little or no change of strength relative to fresh rock.
FR		Fresh	Rock shows no sign of decomposition or staining.

ABBREVIATIONS AND DESCRIPTIONS FOR ROCK MATERIAL AND DEFECTS

CLASSIFICATION AND INFERRED STRATIGRAPHY
Rock is broadly classified and described in Borehole and Test Pit Logs using the preferred method given in AS1726 – 2017, Section 6.2 – Rock identification, description and classification.

DETAILED ROCK DEFECT SPACING

Defect Spacing

Spacing/width (mm)	Descriptor	Symbol
<20	Extremely Close	EC
20-60	Very Close	VC
60-200	Close	C
200-600	Medium	M
600-2000	Wide	W
2000-6000	Very Wide	VW

Bedding Thickness (Stratification)

Term	Spacing (mm)
Thinly laminated	<6
Laminated	6 – 20
Very thinly bedded	20 – 60
Thinly bedded	60 – 200
Medium bedded	200 – 600
Thickly bedded	600 – 2,000
Very thickly bedded	> 2,000

ABBREVIATIONS AND DESCRIPTIONS FOR DEFECT TYPES

Defect Type	Abbr.	Description
Joint	JT	Surface of a fracture or parting, formed without displacement, across which the rock has little or no tensile strength. May be closed or filled by air, water or soil or rock substance, which acts as cement.
Bedding Parting	BP	Surface of fracture or parting, across which the rock has little or no tensile strength, parallel or sub-parallel to layering/ bedding. Bedding refers to the layering or stratification of a rock, indicating orientation during deposition, resulting in planar anisotropy in the rock material.
Contact	CO	The surface between two types or ages of rock.
Sheared Surface	SSU	A near planar, curved or undulating surface which is usually smooth, polished or slickensided.
Sheared Seam/ Zone (Fault)	SS/SZ	Seam or zone with roughly parallel almost planar boundaries of rock substance cut by closely spaced (often <50 mm) parallel and usually smooth or slickensided joints or cleavage planes.
Crushed Seam/ Zone (Fault)	CS/CZ	Seam or zone composed of disoriented usually angular fragments of the host rock substance, with roughly parallel near-planar boundaries. The brecciated fragments may be of clay, silt, sand or gravel sizes or mixtures of these.
Extremely Weathered Seam/ Zone	XWS/XWZ	Seam of soil substance, often with gradational boundaries, formed by weathering of the rock material in places.
Infilled Seam	IS	Seam of soil substance, usually clay or clayey, with very distinct roughly parallel boundaries, formed by soil migrating into joint or open cavity.
Vein	VN	Distinct sheet-like body of minerals crystallised within rock through typically open-space filling or crack-seal growth.

NOTE: Defects size of <100mm SS, CS and XWS. Defects size of >100mm SZ, CZ and XWZ.

ABBREVIATIONS AND DESCRIPTIONS FOR DEFECT SHAPE AND ROUGHNESS

Shape	Abbr.	Description	Roughness	Abbr.	Description
Planar	PR	Consistent orientation	Polished	POL	Shiny smooth surface
Curved	CU	Gradual change in orientation	Slickensided	SL	Grooved or striated surface, usually polished
Undulating	UN	Wavy surface	Smooth	SM	Smooth to touch. Few or no surface irregularities
Stepped	ST	One or more well defined steps	Rough	RO	Many small surface irregularities (amplitude generally <1mm). Feels like fine to coarse sandpaper
Irregular	IR	Many sharp changes in orientation	Very Rough	VR	Many large surface irregularities, amplitude generally >1mm. Feels like very coarse sandpaper

Orientation: **Vertical Boreholes –** The dip (inclination from horizontal) of the defect.
Inclined Boreholes – The inclination is measured as the acute angle to the core axis.

ABBREVIATIONS AND DESCRIPTIONS FOR DEFECT COATING

Coating	Abbr.	Description
Clean	CN	No visible coating or infilling
Stain	SN	No visible coating but surfaces are discoloured by staining, often limonite (orange-brown)
Veneer	VNR	A visible coating of soil or mineral substance, usually too thin to measure (< 1 mm); may be patchy

DEFECT APERTURE

Aperture	Abbr.	Description
Closed	CL	Closed.
Open	OP	Without any infill material.
Infilled	-	Soil or rock i.e. clay, silt, talc, pyrite, quartz, etc.

Appendix B – Laboratory Certificates

STS GEOTECHNICS PTY LTD
CONSULTING GEOTECHNICAL ENGINEERS

STS Geotechnics Pty Ltd
14/1 Cowpasture Place, Wetherill Park NSW 2164
Phone: (02)9756 2166 | Email: enquiries@stsgeo.com.au

NATA Accredited for Compliance with ISO/IEC 17025 - Testing No. 2750

Point Load Strength Index Report

Project: E24175.G04, 28 Elizabeth St, Liverpool, NSW
Client: EI Australia Pty Ltd
Address: Suite 6.01, 55 Miller Street, Pyrmont NSW 2009
Test Method: AS4133.4.1

Project No.: 31288/5294D-L
Report No.: 21/1996
Report Date: 1/07/2021

Sampling Procedure: Samples Supplied By Client (Not covered under NATA Scope of Accreditation)

Date Samples Drilled / Taken: 23/06/2021

Borehole No. 101

Depth	Test Type	Is(50) (Mpa)	Rock Type	Failure Type	Moisture
16.68	A	**1.20**	SH	3	D
17.49	A	**1.50**	SH	3	M
18.45	A	**0.90**	SH	3	M
19.59	A	**1.30**	SH	3	D
20.66	A	**1.30**	SH	3	D
21.51	A	**1.50**	SH	3	D

Sampling Procedure: Samples Supplied By Client (Not covered under NATA Scope of Accreditation)

Date Samples Drilled / Taken: 23/06/2021

Borehole No. 102M

Depth	Test Type	Is(50) (Mpa)	Rock Type	Failure Type	Moisture
16.72	A	**1.30**	SH	3	D
17.49	A	**1.40**	SH	3	D
18.50	A	**1.30**	SH	3	D
19.46	A	**0.58**	SH	3	D
20.43	A	**1.70**	SH	3	D
21.35	A	**1.10**	SH	3	D
22.47	A	**1.70**	SH	3	D

FAILURE TYPE
1= FRACTURE THROUGH BEDDING OR WEAK PLANE
2= FRACTURE ALONG BEDDING
3= FRACTURE THROUGH ROCK MASS
4= FRACTURE INFLUENCED BY NATURAL DEFECT OR DRILLING
5= PARTIAL FRACTURE OR CHIP (INVALID RESULT)

TEST TYPE
A= AXIAL
D= DIAMETRAL
I= IRREGULAR
C= CUBE

MOISTURE CONDITION
W= WET
M= MOIST
D= DRY

ROCK TYPE
SS= SANDSTONE
ST= SILTSTONE
SH= SHALE
YS= CLAYSTONE
IG= IGNEOUS

Remarks:

Approved Signatory.....

Technician: FV

STS GEOTECHNICS PTY LTD
CONSULTING GEOTECHNICAL ENGINEERS

STS Geotechnics Pty Ltd
14/1 Cowpasture Place, Wetherill Park NSW 2164
Phone: (02)9756 2166 | Email: enquiries@stsgeo.com.au

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Test Method: AS4133.4.1

Project No.: 31288/5294D-L
Report No.: 21/1996
Report Date: 1/07/2021

Sampling Procedure: Samples Supplied By Client (Not covered under NATA Scope of Accreditation)

Date Samples Drilled / Taken: 23/06/2021

Borehole No. 103

Depth	Test Type	Is(50) (Mpa)	Rock Type	Failure Type	Moisture
17.17	A	**1.30**	SH	3	D
18.46	A	**1.30**	SH	3	D
19.40	A	**2.00**	SH	3	D
20.43	A	**1.30**	SH	3	D

Sampling Procedure: Samples Supplied By Client (Not covered under NATA Scope of Accreditation)

Date Samples Drilled / Taken: 23/06/2021

Borehole No. 104

Depth	Test Type	Is(50) (Mpa)	Rock Type	Failure Type	Moisture
16.61	A	**1.30**	SH	3	D
17.50	A	**1.20**	SH	3	D
18.45	A	**1.20**	SH	3	D
19.69	A	**2.70**	SH	3	D
20.46	A	**1.70**	SH	3	D

FAILURE TYPE
1= FRACTURE THROUGH BEDDING OR WEAK PLANE
2= FRACTURE ALONG BEDDING
3= FRACTURE THROUGH ROCK MASS
4= FRACTURE INFLUENCED BY NATURAL DEFECT OR DRILLING
5= PARTIAL FRACTURE OR CHIP (INVALID RESULT)

TEST TYPE
A= AXIAL
D= DIAMETRAL
I= IRREGULAR
C= CUBE

MOISTURE CONDITION
W= WET
M= MOIST
D= DRY

ROCK TYPE
SS= SANDSTONE
ST= SILTSTONE
SH= SHALE
YS= CLAYSTONE
IG= IGNEOUS

Remarks:

Approved Signatory.....

Technician: FV

STS GEOTECHNICS PTY LTD
CONSULTING GEOTECHNICAL ENGINEERS

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Point Load Strength Index Report

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Test Method: AS4133.4.1

Project No.: 31288/5294D-L
Report No.: 21/1996
Report Date: 1/07/2021
Page: 3 of 3

Sampling Procedure: Samples Supplied By Client (Not covered under NATA Scope of Accreditation)

Date Samples Drilled / Taken: 23/06/2021

Borehole No. 105

Depth	Test Type	Is(50) (Mpa)	Rock Type	Failure Type	Moisture
16.53	A	**0.80**	SH	3	D
17.50	A	**0.97**	SH	3	D
18.52	A	**0.79**	SH	3	D
19.42	A	**0.83**	SH	3	D
20.50	A	**0.87**	SH	3	M
21.45	A	**1.50**	SH	3	D
22.24	A	**2.00**	SH	3	D

Sampling Procedure: Samples Supplied By Client (Not covered under NATA Scope of Accreditation)

Date Samples Drilled / Taken:

Borehole No.

Depth	Test Type	Is(50) (Mpa)	Rock Type	Failure Type	Moisture

FAILURE TYPE
1= FRACTURE THROUGH BEDDING OR WEAK PLANE
2= FRACTURE ALONG BEDDING
3= FRACTURE THROUGH ROCK MASS
4= FRACTURE INFLUENCED BY NATURAL DEFECT OR DRILLING
5= PARTIAL FRACTURE OR CHIP (INVALID RESULT)

TEST TYPE
A= AXIAL
D= DIAMETRAL
I= IRREGULAR
C= CUBE

MOISTURE CONDITION
W= WET
M= MOIST
D= DRY

ROCK TYPE
SS= SANDSTONE
ST= SILTSTONE
SH= SHALE
YS= CLAYSTONE
IG= IGNEOUS

Remarks:

Approved Signatory.....

Technician: FV

Appendix C – Vibration Limits

German Standard DIN 4150 – Part 3: 1999 provides guideline levels of vibration velocity for evaluating the effects of vibration in structures. The limits presented in this standard are generally considered to be conservative.

The DIN 4150 values (maximum levels measured in any direction at the foundation, OR, maximum levels measured in (x) or (y) directions, in the plane of the uppermost floor), are summarised in **Table A** below.

It should be noted that peak vibration velocities higher than the minimum figures in **Table A** for low frequencies may be quite 'safe', depending on the frequency content of the vibration and the actual conditions of the structures.

It should also be noted that these levels are 'safe limits', up to which no damage due to vibration effects has been observed for the particular class of building. 'Damage' is defined by DIN 4150 to include even minor non-structural cracking in cement render, the enlargement of cracks already present, and the separation of partitions or intermediate walls from load bearing walls. Should damage be observed at vibration levels lower than the 'safe limits', then it may be attributed to other causes. DIN 4150 also states that when vibration levels higher than the 'safe limits' are present, it does not necessarily follow that damage will occur. Values given are only a broad guide.

Table A DIN 4150 – Structural Damage – Safe Limits for Building Vibration

Group	Type of Structure	Peak Vibration Velocity (mm/s)			
		At Foundation Level at a Frequency of:			Plane of Floor of Uppermost Storey
		Less than 10 Hz	10 Hz to 50 Hz	50 Hz to 100 Hz	All Frequencies
1	Buildings used for commercial purposes, industrial buildings and buildings of similar design	20	20 to 40	40 to 50	40
2	Dwellings and buildings of similar design and/or use	5	5 to 15	15 to 20	15
3	Structures that because of their particular sensitivity to vibration, do not correspond to those listed in Group 1 and 2 and have intrinsic value (e.g. buildings that are under a preservation order)	3	3 to 8	8 to 10	8

Note: For frequencies above 100 Hz, the higher values in the 50 Hz to 100 Hz column should be used.

Appendix D – Important Information

Important Information

SCOPE OF SERVICES

The geotechnical report ("the report") has been prepared in accordance with the scope of services as set out in the contract, or as otherwise agreed, between the Client And EI Australia ("EI"). The scope of work may have been limited by a range of factors such as time, budget, access and/or site disturbance constraints.

RELIANCE ON DATA

EI has relied on data provided by the Client and other individuals and organizations, to prepare the report. Such data may include surveys, analyses, designs, maps and plans. EI has not verified the accuracy or completeness of the data except as stated in the report. To the extent that the statements, opinions, facts, information, conclusions and/or recommendations ("conclusions") are based in whole or part on the data, EI will not be liable in relation to incorrect conclusions should any data, information or condition be incorrect or have been concealed, withheld, misrepresented or otherwise not fully disclosed to EI.

GEOTECHNICAL ENGINEERING

Geotechnical engineering is based extensively on judgment and opinion. It is far less exact than other engineering disciplines. Geotechnical engineering reports are prepared for a specific client, for a specific project and to meet specific needs, and may not be adequate for other clients or other purposes (e.g. a report prepared for a consulting civil engineer may not be adequate for a construction contractor). The report should not be used for other than its intended purpose without seeking additional geotechnical advice. Also, unless further geotechnical advice is obtained, the report cannot be used where the nature and/or details of the proposed development are changed.

LIMITATIONS OF SITE INVESTIGATION

The investigation programme undertaken is a professional estimate of the scope of investigation required to provide a general profile of subsurface conditions. The data derived from the site investigation programme and subsequent laboratory testing are extrapolated across the site to form an inferred geological model, and an engineering opinion is rendered about overall subsurface conditions and their likely behaviour with regard to the proposed development. Despite investigation, the actual conditions at the site might differ from those inferred to exist, since no subsurface exploration program, no matter how comprehensive, can reveal all subsurface details and anomalies. The engineering logs are the subjective interpretation of subsurface conditions at a particular location and time, made by trained personnel. The actual interface between materials may be more gradual or abrupt than a report indicates.

SUBSURFACE CONDITIONS ARE TIME DEPENDENT

Subsurface conditions can be modified by changing natural forces or man-made influences. The report is based on conditions that existed at the time of subsurface exploration. Construction operations adjacent to the site, and natural events such as floods, or ground water fluctuations, may also affect subsurface conditions, and thus the continuing adequacy of a geotechnical report. EI should be kept appraised of any such events, and should be consulted to determine if any additional tests are necessary.

VERIFICATION OF SITE CONDITIONS

Where ground conditions encountered at the site differ significantly from those anticipated in the report, either due to natural variability of subsurface conditions or construction activities, it is a condition of the report that EI be notified of any variations and be provided with an opportunity to review the recommendations of this report. Recognition of change of soil and rock conditions requires experience and it is recommended that a suitably experienced geotechnical engineer be engaged to visit the site with sufficient frequency to detect if conditions have changed significantly.

REPRODUCTION OF REPORTS

This report is the subject of copyright and shall not be reproduced either totally or in part without the express permission of this Company. Where information from the accompanying report is to be included in contract documents or engineering specification for the project, the entire report should be included in order to minimize the likelihood of misinterpretation from logs.

REPORT FOR BENEFIT OF CLIENT

The report has been prepared for the benefit of the Client and no other party. EI assumes no responsibility and will not be liable to any other person or organisation for or in relation to any matter dealt with or conclusions expressed in the report, or for any loss or damage suffered by any other person or organisation arising from matters dealt with or conclusions expressed in the report (including without limitation matters arising from any negligent act or omission of EI or for any loss or damage suffered by any other party relying upon the matters dealt with or conclusions expressed in the report). Other parties should not rely upon the report or the accuracy or completeness of any conclusions and should make their own inquiries and obtain independent advice in relation to such matters.

OTHER LIMITATIONS

EI will not be liable to update or revise the report to take into account any events or emergent circumstances or fact occurring or becoming apparent after the date of the report.